

# Traffic Impact Statement / Study

## Clark Street Distribution Center

3301 Clark Street  
Tampa, Florida 33605

Prepared For:  
Keating Acquisitions, LLC  
&  
City of Tampa

Prepared By:  
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May 4, 2018





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## Introduction:

Keating Acquisitions, LLC intends to build a distribution warehouse for the recently purchased undeveloped land located at 3301 Clark St., Tampa, Florida. The proposed project will consist of a 179,080 square feet warehouse and 289 parking spaces within the existing land boundary, as displayed in Figure 1. The site plan of the project can be found in the Appendix.



Figure 1: Site Location<sup>1</sup>

The purpose of this Traffic Impact Study (TIS) is to assess the existing level of service (LOS) of the roads and intersections that will be affected by the proposed development and compare these levels of service with proposed future levels of service at these roads and intersections, and establish any necessary transportation mitigation measures that may be needed within the immediate area of the proposed project. This project's affected area includes three signalized intersections (State Road 60 & 34<sup>th</sup> Street, State Road 60 & 22<sup>nd</sup> Street, and State Road 60 & 21<sup>st</sup> Street), one unsignalized intersection (Clark Street & 34<sup>th</sup> Street).

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<sup>1</sup> Google Earth image



## Existing Roadway Characteristics:

### Clark Street, between 30<sup>th</sup> Street and 34<sup>th</sup> Street

The existing roadway is two-lane, undivided local road in an industrial area. Characteristics the roadway lacks are roadway lighting and sidewalks. Additionally, the roadway lacks a paved shoulder and pavement markings. Curb and gutter exists along S. 34<sup>th</sup> St. and terminates at the intersection with Clark St. Drainage inlets and manholes are along both sides of Clark St., west of the project area. There is no posted speed limit along this segment.

### Clark Street and 34<sup>th</sup> Street intersection

The intersection at Clark Street and 34<sup>th</sup> Street is an unsignalized intersection. Existing traffic control devices includes STOP sign along Clark Street, however there are no pavement markings. 34<sup>th</sup> Street is a four lane highway with a four foot traffic separator. The east approach of the intersection provides ingress and egress for another parcel. It is assumed that no project traffic will use this approach.

### 34th Street, between State Road 60 and Clark Street

As previously stated, 34<sup>th</sup> Street is a four lane highway with a four foot traffic separator. The existing posted speed limit is 30 mph. The roadway's AADT<sup>2</sup> is approximately 3600 vehicles. Lighting exists along the west side of the roadway. Curb and gutter drainage is present throughout the length of roadway affected by this project. Due to the industrial location, there are access drives along this roadway: three on the west, and three on the east. To the north of the roadway, a mid-block crosswalk is present for the bike trail.

### State Road 60 and 34<sup>th</sup> Street intersection

At the intersection of State Road 60 (Adamo Drive) and 34<sup>th</sup> Street, the northbound approach consists of one 330-foot left turn lane, one through lane, and one through/right turn lane; the southbound approach consists of one 230-foot left turn lane, one through lane, and one right turn lane; the eastbound approach consists of one 395-foot left turn lane, two through lanes, and one 345-foot right turn lane with 5-foot wide bike lane; the westbound approach consists of one 395-foot left turn lane, two through lanes and one 515-foot right lane with a 5-foot wide bike lane. Along 34<sup>th</sup> Street, the roadway maintains similar characteristics previously mentioned. Roadway characteristics of State Road 60 that are present are curb and gutter drainage to the east with 6-foot wide sidewalks, and 22-foot median with drainage inlets. Roadway lighting is not present at this intersection along State Road 60.

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<sup>2</sup> Florida Department of Transportation Data and Analytics Office, Florida Transportation Information, 2016.



## State Road 60 and 22<sup>nd</sup> Street/21<sup>st</sup> Street intersections

At the intersections at State Road 60 and 22<sup>nd</sup> Street and State Road 60 and 21<sup>st</sup> Street, State Road 60 accommodates eastbound and westbound traffic; northbound traffic travels along 22<sup>nd</sup> Street, and southbound traffic travels along 21<sup>st</sup> Street. At 22<sup>nd</sup> Street, State Road 60 consists of four through lanes, which become two left turn lanes and two through lanes at 21<sup>st</sup> Street and one right turn lane into 22<sup>nd</sup> Street. 22<sup>nd</sup> Street consists of one left turn lane, one through-left lane, two through lanes, and one right turn lane.

### Level of Service Analysis

Level of service (LOS) analysis was performed using HCS software, based the Highway Capacity Manual 2010 methodology.

Table 1:

EXISTING LEVEL OF SERVICE					
Intersection	Approach	AM Peak		PM Peak	
		Delay (s)	LOS	Delay (s)	LOS
34th Street/ Clark Street	NB	--	--	--	--
	SB	--	--	--	--
	EB	11.1	B	10.3	B
	WB	9.3	A	9.1	A
State Road 60/ 34th Street	NB	104.2	F	103.3	F
	SB	112.9	F	195.5	F
	EB	28.5	C	46.0	D
	WB	34.9	C	36.9	C
Overall		43.6	D	53.3	D
State Road 60/ 22nd Street	NB	68.6	E	84.7	F
	SB	--	--	--	--
	EB	9.8	A	16.9	B
	WB	21.0	C	18.2	B
Overall		44.2	D	41.9	D
State Road 60/ 21st Street	NB	--	--	--	--
	SB	44.1	D	49.3	D
	EB	30.9	C	42.5	D
	WB	18.4	B	19.7	B
Overall		28.0	C	35.5	D

From the existing level of service analysis, an excessive delay exists along the northbound and southbound approaches of 34<sup>th</sup> Street at State Road 60, due to extreme demand along the westbound approach in the AM peak hour and the eastbound and westbound approaches in the PM peak hour. Traffic counts and HCS reports are located in the Appendix.



## Future Roadway Characteristics:

For the purpose of this analysis, a 2% growth rate is applied to existing traffic counts with a build out year of 2020 to obtain the future background traffic for the subject area. There are no capital improvement projects scheduled for construction within the affected roadway network, for both the City of Tampa<sup>3</sup> and Florida Department of Transportation<sup>4</sup> that would create additional demand or capacity. The level of service of the future background traffic will be compared the level of service of the future background traffic plus the project traffic.

Table 2:

FUTURE BACKGROUND LEVEL OF SERVICE					
Intersection	Approach	AM Peak		PM Peak	
		Delay (s)	LOS	Delay (s)	LOS
34th Street/ Clark Street	NB	--	--	--	--
	SB	--	--	--	--
	EB	11.3	B	10.5	B
	WB	9.5	A	9.1	A
State Road 60/ 34th Street	NB	106.4	F	103.5	F
	SB	117.2	F	243.5	F
	EB	29.2	C	47.1	D
	WB	35.6	D	37.1	D
Overall		42.2	D	57.1	E
State Road 60/ 22nd Street	NB	111.2	F	110.7	F
	SB	--	--	--	--
	EB	10.1	B	17.2	B
	WB	21.7	C	18.4	B
Overall		55.5	E	51.5	D
State Road 60/ 21st Street	NB	--	--	--	--
	SB	44.8	D	50.7	D
	EB	31.0	C	43.8	D
	WB	19.3	B	20.6	C
Overall		28.9	C	36.8	D

Trip generation calculations were performed using Institute of Transportation Engineers' (ITE) *Trip Generation, 9<sup>th</sup> edition*. The following assumptions were made, regarding entering and exiting traffic within the peak hours:

1. In order to perform analysis for a worst case, it is assumed that all traffic entering and exiting the site shall be trucks and will contribute to additional heavy vehicle percent in determining the level of service at each intersection.

<sup>3</sup> <https://www.tampagov.net/tss-transportation/info/projects>

<sup>4</sup> <http://www2.dot.state.fl.us/fmsupportapps/workprogram/WorkProgram.aspx>



2. To eliminate project trips within the adjacent neighborhood to the west, a “NO TRUCKS” sign shall be placed westbound along Clark Street, prior to 30<sup>th</sup> Street. Additionally, “NO TRUCKS” signs with left directional arrow may be placed along Clark Street, facing the project driveways, to further discourage westbound project traffic along Clark Street.

The estimated trip ends for AM peak hour, PM peak hour, and daily trips are summarized in Table 3.

Table 3: Project Trip Generation

<u>Land Use</u>	<u>ITE LUC</u>	<u>Size</u>	<u>Daily Trips</u>	AM Peak Hour Trip Ends			PM Peak Hour Trip Ends		
				In	Out	Total	In	Out	Total
High Cube Warehouse/ Distribution Center	152	180 KSF	302	14	6	20	7	15	22

Table 4:

FUTURE BACKGROUND + PROJECT LEVEL OF SERVICE						
Intersection	Approach	AM Peak		PM Peak		
		Delay (s)	LOS	Delay (s)	LOS	
34th Street/ Clark Street	NB	--	--	--	--	
	SB	--	--	--	--	
	EB	11.6	B	11.1	B	
	WB	9.8	A	9.2	A	
State Road 60/ 34th Street	NB	111.6	F	104.9	F	
	SB	117.2	F	251.1	F	
	EB	29.1	C	47.0	D	
	WB	35.5	D	37.1	C	
Overall		42.4	D	57.8	E	
State Road 60/ 22nd Street	NB	116.7	F	110.7	F	
	SB	--	--	--	--	
	EB	10.1	A	16.9	B	
	WB	21.7	C	18.4	B	
Overall		69.8	E	51.8	D	
State Road 60/ 21st Street	NB	--	--	--	--	
	SB	44.8	D	50.7	D	
	EB	31.0	C	43.8	D	
	WB	19.3	B	20.6	C	
Overall		28.9	C	36.8	D	

The delay at the unsignalized intersection at Clark Street and 34<sup>th</sup> Street increased an average of 0.5 seconds. The signalized intersections of State Road 60 and 34 Street, 22<sup>nd</sup> Street, and 21<sup>st</sup> Street, respectively increased 0.2, 14.3, and 0 seconds in the AM peak hour and 0.6, 0.3, and 0



seconds in the PM peak hour. Due to the relatively low volume of demand generated from the project compared to the east–west volume along State Road 60 (Adamo Drive), no geometry changes should be made the roadway network of this project.

## Study Conclusions:

- The Clark Street Distribution Center’s traffic will have little impact to delay at existing intersections within the project area.
- No roadway geometry changes will be necessary to accommodate the trips generated from this project.
- “NO TRUCKS” sign will be placed along Clark Street to eliminate disruption to adjacent neighborhood.



## APPENDIX



George F. Young, Inc.

1915

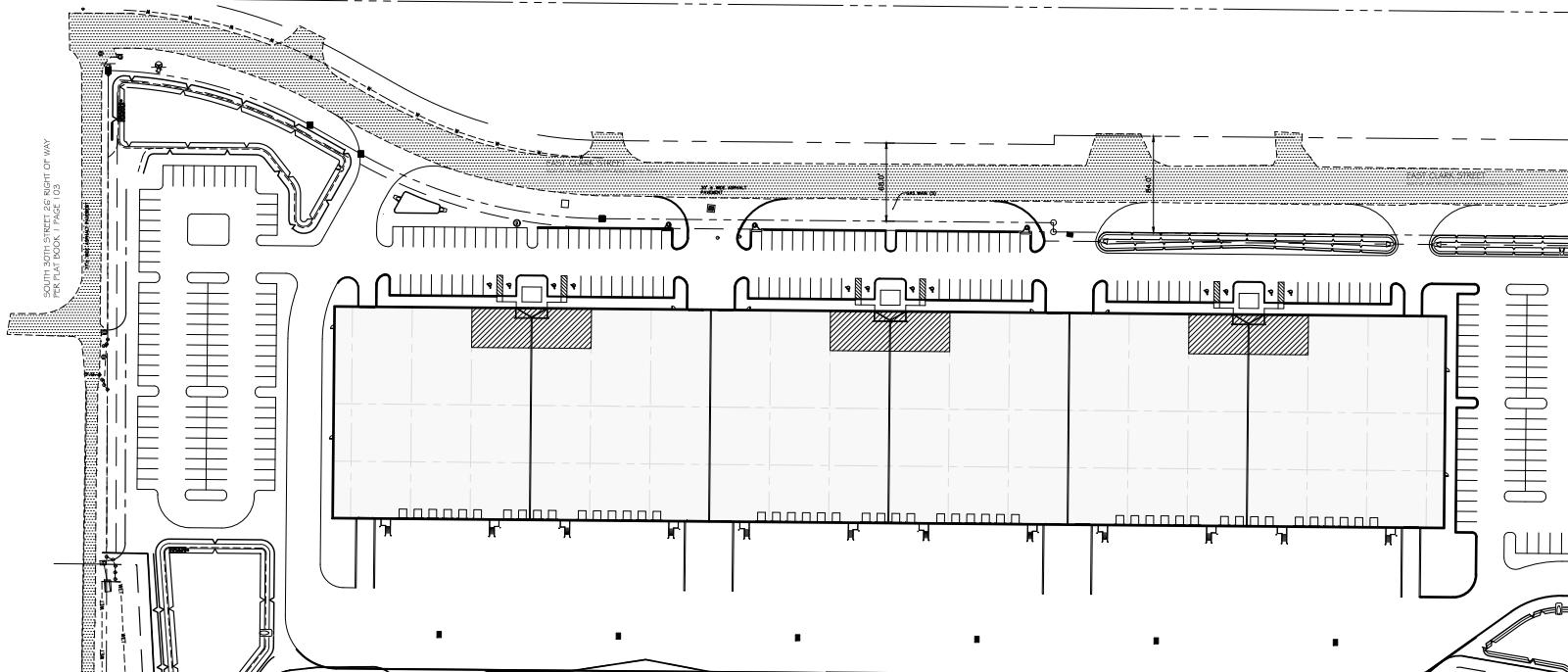
PHONE: (813) 223-5000 FAX: (813) 223-5005  
WEBSITE: WWW.GFYINC.COM  
EMAIL: INFO@GFYINC.COM  
CIV. & TRANSPORTATION ENGINEERING CONSULTANTS  
PLANNING & DESIGNING FOR A BETTER TOMORROW

50' 50'

SCALE: 0' - 50'



SOUTH 34TH STREET, RIGHT OF WAY AND LANDOWNING



CITY OF TAMPA SITE DATA TABLE

TOTAL CONTINUOUS SITE AREA	703,494 S.F. / 16.15 ACRES
PROPOSED SF # OF BUILDINGS	179,080 S.F., 1 BUILDING
TYPE OF CONSTRUCTION	IIB
FINISHED FLOOR ELEVATION	11.00 (NAVD88)
PROPOSED BUILDING HEIGHT	45'-0"
NUMBER OF FLOORS	1
EXISTING OCCUPANCY CLASS	VACANT
COULD BE OCCUPIED	N/A
NUMBER OF UNITS (RESIDENTIAL USE) - PROPOSED	N/A
DENSITY (RESIDENTIAL USE) - ALLOWED	N/A
MINIMUM SETBACKS	NORTH 8', SOUTH 0', EAST 8' WEST 8'
ASSUMED PROPERTY LINES	PERMIT ISSUED
PROPOSED BUILDING USE	DISTRIBUTION WAREHOUSE
PARKING REQUIRED	0.6 PER EMPLOYEE ON LARGEST SHIFT
PARKING PROVIDED	289 (INCLUDING 12 HANDICAP SPACES)
H/C PARKING REQUIRED	9
H/C PARKING PROVIDED	12
EXISTING SITE IMPERVIOUS AREA	0 SQ. FT
PROPOSED SITE IMPERVIOUS AREA	434,277 SQ. FT
EXISTING PAVED VUA (VEHICULAR USE AREA)	0 SQ. FT
PROPOSED PAVED VUA (VEHICULAR USE AREA)	124,500 SQ. FT
REQUIRED 20% VUA GREEN SPACE	24,900 SQ. FT
PROPOSED 20% VUA GREEN SPACE	25,835 SQ. FT
REQUIRED MULTI-FAMILY / TOWNHOUSE GREEN SPACE	N/A
PROVIDED MULTI-FAMILY / TOWNHOUSE GREEN SPACE	N/A
CURRENT ZONING	IG
LAND USE	VACANT
POLO NUMBER	17800-2
FLOOR AREA RATIO (FAR)	0.25
FEMA PANEL NUMBER	12057C0358H
POTABLE WATER	CITY OF TAMPA
SANITARY SEWER	CITY OF TAMPA
FIRE PROTECTION	PRIVATE & PUBLIC
STORMWATER	CITY OF TAMPA
FIRE RESISTANCE RATING OF EXTERIOR WALLS	
MAXIMUM AREA OF EXTERIOR WALL OPENINGS	
ACCESSIBLE ROUTE ON SITE	YES
LOCATION AND ELEVATION OF A/C UNITS	

CLARK STREET DISTRIBUTION CENTER		OVERALL SITE PLAN	
SECTION NO.	JOB NO.	INSTR.	DATE
SECTION 20 - TOWNSHIP 29 S., RANGE 19 E.	17001700TC	BW - KEATING ACQUISITIONS, LLC	05/20/03
		DRW - 9128 STRATA PLACE, SUITE 1015 NAPLES, FL 34108-4115	
		CHD - NO. 7071 BRIEF & DAVIS	
		DATE - 05/20/03	
		SCALE -	

FILE NUMBER: 17001700TC-05/20/03

PAGE 1

1

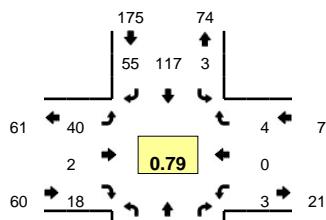
1

Type of peak hour being reported: Intersection Peak

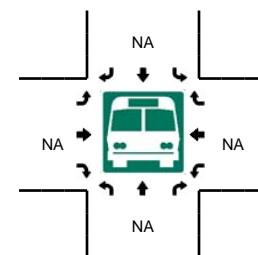
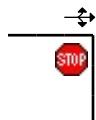
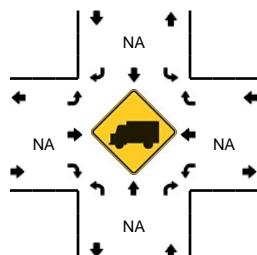
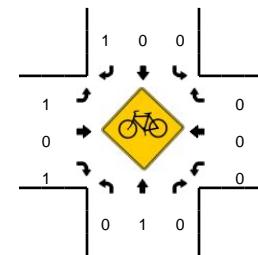
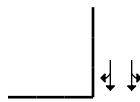
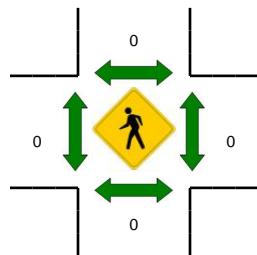
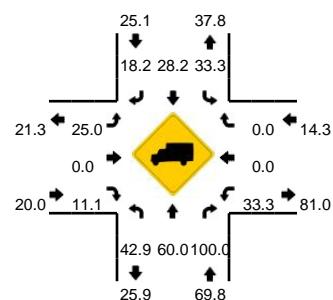
Method for determining peak hour: Total Entering Volume

**LOCATION:** 34th St -- Clark St  
**CITY/STATE:** Tampa, FL

**QC JOB #:** 14664801  
**DATE:** Thu, Mar 29 2018



**Peak-Hour: 7:15 AM -- 8:15 AM**  
**Peak 15-Min: 7:30 AM -- 7:45 AM**



15-Min Count Period Beginning At	34th St (Northbound)					34th St (Southbound)					Clark St (Eastbound)					Clark St (Westbound)					Total	Hourly Totals
	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*		
7:00 AM	0	2	2	0	0	3	20	7	0	0	4	0	1	0	0	0	0	2	0	0	41	
7:15 AM	0	6	3	0	0	1	27	16	0	0	7	0	5	0	0	0	0	0	0	0	65	
7:30 AM	2	6	8	1	0	1	35	20	0	0	13	0	3	0	0	1	0	3	0	0	93	
7:45 AM	2	7	4	0	0	1	43	16	0	0	8	0	9	0	0	0	0	0	0	0	90	289
8:00 AM	2	11	1	0	0	0	12	3	0	0	12	2	1	0	0	2	0	1	0	0	47	295
8:15 AM	2	9	4	0	0	0	11	6	0	0	9	0	0	0	0	0	0	1	0	0	42	272
8:30 AM	1	9	0	0	0	1	18	8	0	0	11	0	1	0	0	0	0	0	0	0	49	228
8:45 AM	0	12	1	0	0	0	17	11	0	0	8	1	0	0	0	1	0	1	0	0	52	190
<b>Peak 15-Min Flowrates</b>																						
<b>Peak 15-Min Flowrates</b>		<b>Northbound</b>				<b>Southbound</b>				<b>Eastbound</b>				<b>Westbound</b>				<b>Total</b>				
		Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	R*
All Vehicles		8	24	32	4	0	4	140	80	0	0	52	0	12	0	0	4	0	12	0	0	372
Heavy Trucks		4	16	32			4	36	12		4	0	0			4	0	0			112	
Pedestrians		0																			0	
Bicycles		0	0	0			0	0	0		0	0	0			0	0	0			0	
Railroad																						
Stopped Buses																						

*Comments:*

Report generated on 4/12/2018 6:27 AM

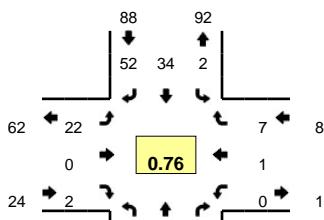
SOURCE: Quality Counts, LLC (<http://www.qualitycounts.net>) 1-877-580-2212

Type of peak hour being reported: Intersection Peak

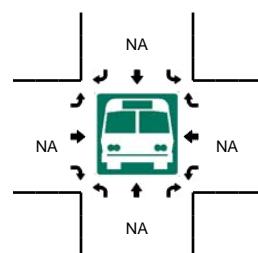
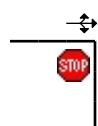
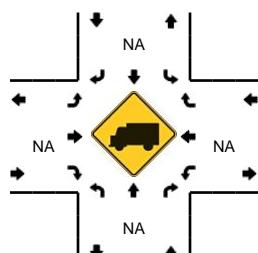
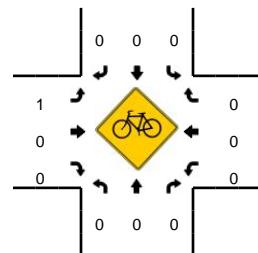
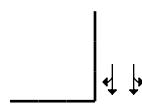
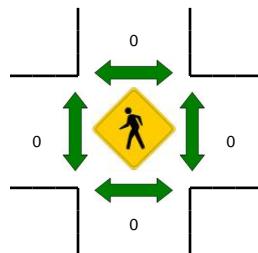
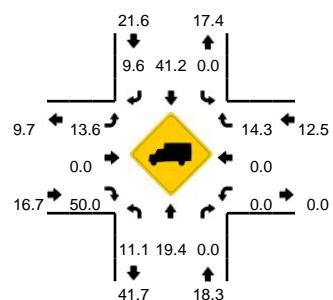
Method for determining peak hour: Total Entering Volume

**LOCATION:** 34th St -- Clark St  
**CITY/STATE:** Tampa, FL

**QC JOB #:** 14664803  
**DATE:** Thu, Mar 29 2018



**Peak-Hour: 4:00 PM -- 5:00 PM**  
**Peak 15-Min: 4:30 PM -- 4:45 PM**



R\* = RTOR

15-Min Count Period Beginning At	34th St (Northbound)				34th St (Southbound)				Clark St (Eastbound)				Clark St (Westbound)				Total	Hourly Totals
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U		
4:00 PM	0	12	0	0	0	10	10	0	0	7	0	1	0	0	0	0	43	
4:15 PM	2	18	0	0	0	10	9	0	0	6	0	1	0	0	0	1	48	
4:30 PM	3	24	0	0	0	1	5	20	0	0	7	0	0	0	0	0	63	
4:45 PM	4	8	0	0	0	9	13	1	0	2	0	0	0	0	0	0	37	191
5:00 PM	1	17	0	0	0	6	12	1	0	3	0	0	0	0	0	1	41	189
5:15 PM	1	4	0	0	0	9	18	0	0	12	0	1	0	0	0	1	46	187
5:30 PM	1	17	0	1	0	6	19	0	0	9	1	0	0	0	0	2	56	180
5:45 PM	3	13	0	0	0	7	13	0	0	9	0	2	0	0	0	1	48	191

Peak 15-Min Flowrates	Northbound					Southbound					Eastbound					Westbound					Total
	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	
All Vehicles	12	96	0	0	0	4	20	80	0	0	28	0	0	0	0	0	0	12	0	0	252
Heavy Trucks	4	12	0	0	0	0	8	8	0	0	4	0	0	0	0	0	0	4	0	0	40
Pedestrians	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Railroad	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Stopped Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Comments:

Report generated on 4/12/2018 6:27 AM

SOURCE: Quality Counts, LLC (<http://www.qualitycounts.net>) 1-877-580-2212

Type of peak hour being reported: Intersection Peak

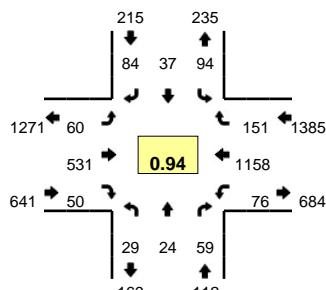
Method for determining peak hour: Total Entering Volume

**LOCATION:** 34th St -- SR 60

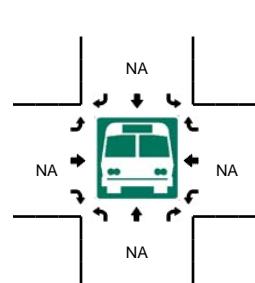
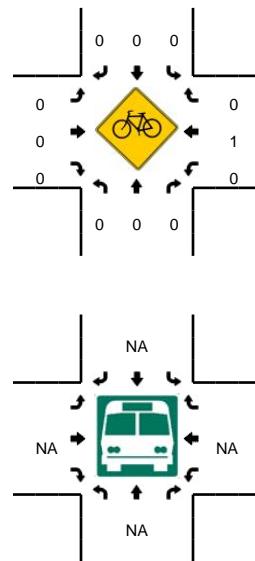
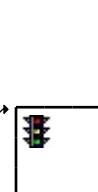
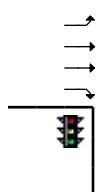
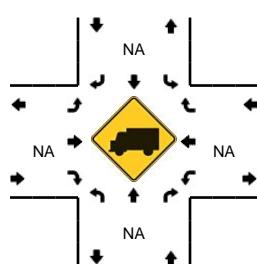
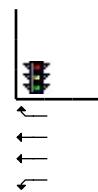
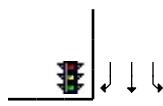
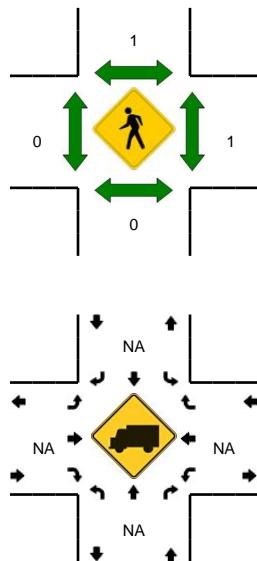
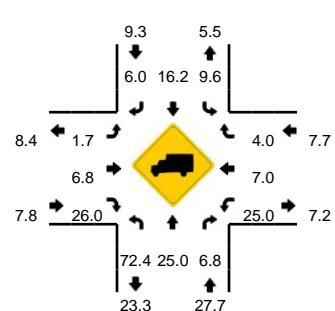
**CITY/STATE:** Tampa, FL

**QC JOB #:** 14664804

**DATE:** Thu, Apr 05 2018



**Peak-Hour: 7:15 AM -- 8:15 AM**  
**Peak 15-Min: 7:45 AM -- 8:00 AM**



R\* = RTOR

15-Min Count Period Beginning At	34th St (Northbound)					34th St (Southbound)					SR 60 (Eastbound)					SR 60 (Westbound)					Total	Hourly Totals
	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*		
7:00 AM	3	3	0	0	6	28	5	7	0	12	14	102	7	2	2	15	292	23	0	6	527	
7:15 AM	10	4	1	0	13	21	7	6	0	16	12	106	9	0	2	25	307	32	0	8	579	
7:30 AM	4	7	3	0	8	29	14	5	0	8	11	138	7	0	5	22	312	28	0	13	614	
7:45 AM	7	5	4	0	20	28	12	6	0	28	23	151	10	0	6	15	275	29	0	8	627	2347
8:00 AM	8	8	2	0	8	16	4	2	0	13	14	136	7	0	4	14	264	28	0	5	533	2353
8:15 AM	8	4	1	0	12	24	10	1	0	10	17	130	7	0	6	17	296	30	0	6	579	2353
8:30 AM	6	5	0	0	13	22	8	4	0	12	23	107	6	0	3	10	267	12	0	5	503	2242
8:45 AM	9	5	4	0	6	12	5	1	0	20	7	114	8	0	4	14	267	7	0	6	489	2104

Peak 15-Min Flowrates	Northbound					Southbound					Eastbound					Westbound					Total
	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	
All Vehicles	28	20	16	0	80	112	48	24	0	112	92	604	40	0	24	60	1100	116	0	32	2508
Heavy Trucks	24	4	4			8	4	4			0	40	24			16	92	4			224
Pedestrians	0					0					0					0					0
Bicycles	0	0	0			0	0	0			0	0	0			0	1	0			1
Railroad																					
Stopped Buses																					

*Comments:*

Type of peak hour being reported: Intersection Peak

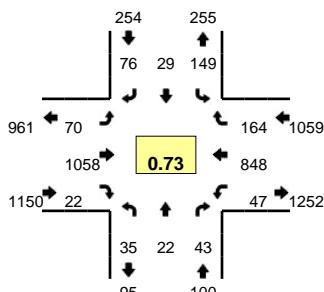
Method for determining peak hour: Total Entering Volume

**LOCATION:** 34th St -- SR 60

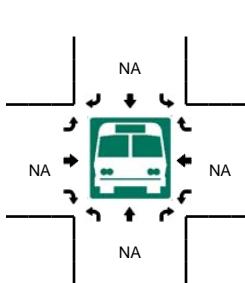
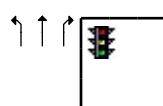
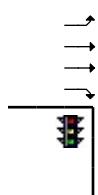
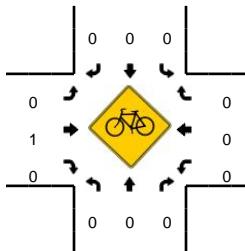
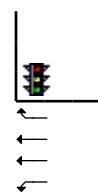
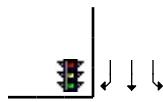
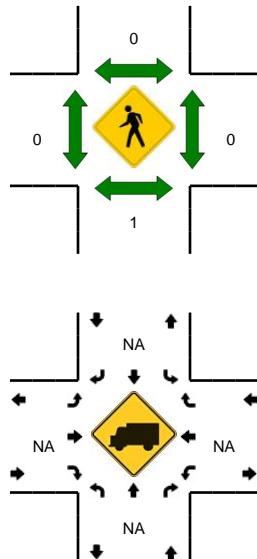
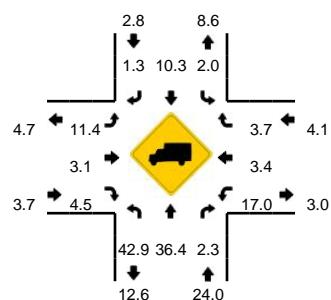
**CITY/STATE:** Tampa, FL

**QC JOB #:** 14664806

**DATE:** Thu, Apr 05 2018



**Peak-Hour: 4:45 PM -- 5:45 PM**  
**Peak 15-Min: 5:15 PM -- 5:30 PM**



15-Min Count Period Beginning At	34th St (Northbound)					34th St (Southbound)					SR 60 (Eastbound)					SR 60 (Westbound)					<b>Total</b>	<b>Hourly Totals</b>
	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*		
4:00 PM	11	9	8	0	10	29	6	3	0	10	19	255	5	1	2	21	203	17	0	6	615	
4:15 PM	26	11	2	0	23	25	3	0	0	18	14	267	10	0	0	11	189	23	1	7	630	
4:30 PM	13	3	3	0	16	27	8	2	0	13	22	269	4	0	1	12	185	20	1	3	602	
4:45 PM	14	3	0	0	8	40	4	3	1	18	14	261	4	1	0	11	193	37	0	11	623	2470
5:00 PM	6	6	1	0	21	43	13	10	0	9	13	128	6	0	2	8	144	49	2	1	462	2317
5:15 PM	8	9	1	0	4	41	9	5	0	20	25	394	5	0	0	14	301	30	0	9	875	2562
5:30 PM	7	4	1	0	7	24	3	1	0	10	16	275	5	1	0	11	210	19	1	8	603	2563
5:45 PM	11	0	0	0	11	23	2	0	0	23	13	220	5	0	0	7	180	25	0	3	523	2463

Peak 15-Min Flowrates	Northbound					Southbound					Eastbound					Westbound					<b>Total</b>
	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	
All Vehicles	32	36	4	0	16	164	36	20	0	80	100	1576	20	0	0	56	1204	120	0	36	3500
Heavy Trucks	16	8	0			4	4	0			8	44	0			12	48	0			144
Pedestrians	0					0					0					0					0
Bicycles	0	0	0			0	0	0			0	1	0			0	0	0			1
Railroad																					
Stopped Buses																					

*Comments:*

Type of peak hour being reported: Intersection Peak

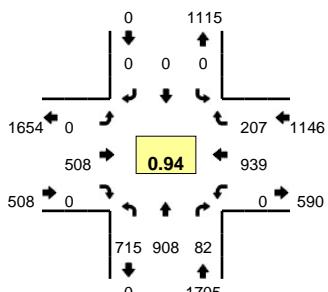
Method for determining peak hour: Total Entering Volume

**LOCATION:** 22nd St -- SR 60

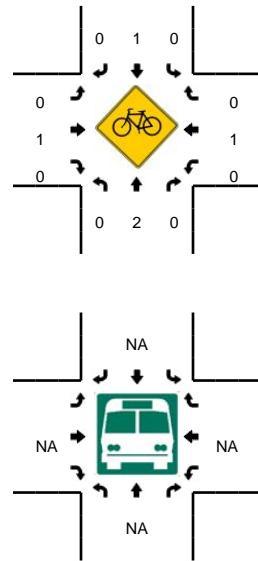
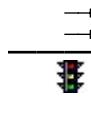
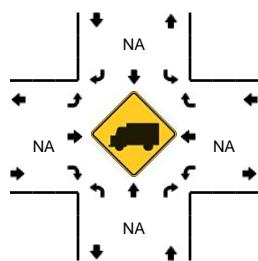
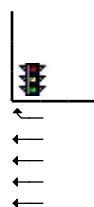
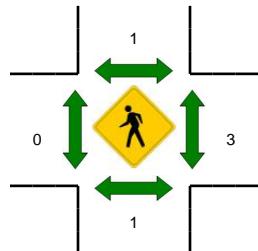
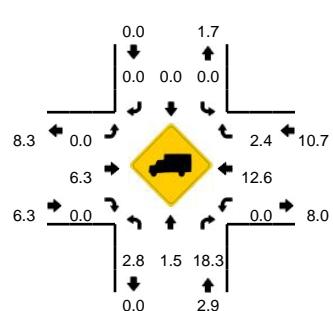
**CITY/STATE:** Tampa, FL

**QC JOB #:** 14664807

**DATE:** Thu, Apr 05 2018



**Peak-Hour: 7:00 AM -- 8:00 AM**  
**Peak 15-Min: 7:45 AM -- 8:00 AM**



R\* = RTOR

15-Min Count Period Beginning At	22nd St (Northbound)					22nd St (Southbound)					SR 60 (Eastbound)					SR 60 (Westbound)					Total	Hourly Totals
	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*		
7:00 AM	197	239	12	0	17	0	0	0	0	0	0	95	0	0	0	0	199	21	0	33	813	
7:15 AM	163	205	5	0	10	0	0	0	0	0	0	107	0	0	0	0	235	23	0	31	779	
7:30 AM	174	224	15	0	1	0	0	0	0	0	0	150	0	0	0	0	265	27	0	18	874	
7:45 AM	181	240	20	0	2	0	0	0	0	0	0	156	0	0	0	0	240	26	0	28	893	3359
8:00 AM	172	224	17	0	4	0	0	0	0	0	0	122	0	0	0	0	202	20	0	26	787	3333
8:15 AM	133	185	15	0	14	0	0	0	0	0	0	141	0	0	0	0	240	17	0	24	769	3323
8:30 AM	179	212	16	0	20	0	0	0	0	0	0	104	0	0	0	0	193	60	0	0	784	3233
8:45 AM	204	199	4	0	15	0	0	0	0	0	0	130	0	0	0	0	201	35	0	11	799	3139

Peak 15-Min Flowrates	Northbound					Southbound					Eastbound					Westbound					Total	
	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*		
All Vehicles	724	960	80	0	8	0	0	0	0	0	0	624	0	0	0	0	960	104	0	112	3572	
Heavy Trucks	32	16	16	0	0	0	0	0	0	0	0	48	0	0	0	0	140	16	0	0	268	
Pedestrians	4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4	
Bicycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	1	
Railroad	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Stopped Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	

*Comments:*

Report generated on 4/12/2018 6:27 AM

SOURCE: Quality Counts, LLC (<http://www.qualitycounts.net>) 1-877-580-2212

Type of peak hour being reported: Intersection Peak

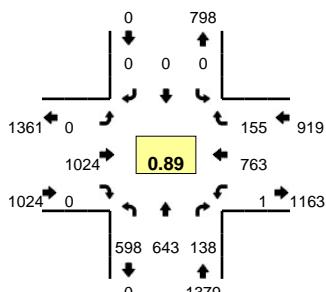
Method for determining peak hour: Total Entering Volume

**LOCATION:** 22nd St -- SR 60

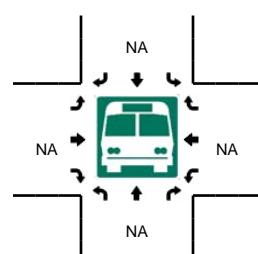
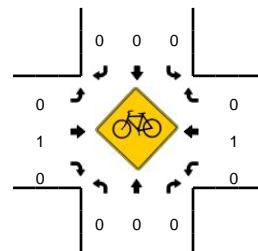
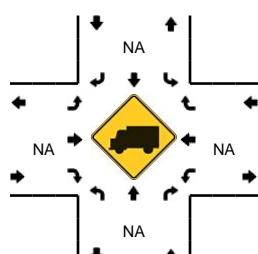
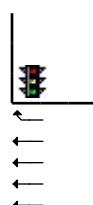
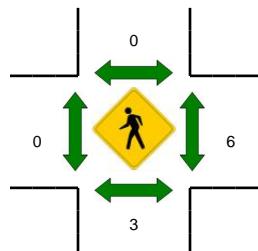
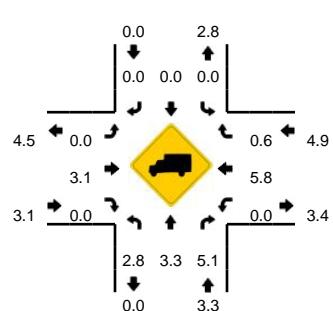
**CITY/STATE:** Tampa, FL

**QC JOB #:** 14664809

**DATE:** Thu, Apr 05 2018



**Peak-Hour: 4:45 PM -- 5:45 PM**  
**Peak 15-Min: 5:15 PM -- 5:30 PM**



R\* = RTOR

15-Min Count Period Beginning At	22nd St (Northbound)					22nd St (Southbound)					SR 60 (Eastbound)					SR 60 (Westbound)					<b>Total</b>	<b>Hourly Totals</b>
	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*		
4:00 PM	151	171	21	0	30	0	0	0	0	0	1	243	0	0	0	0	179	25	0	18	839	
4:15 PM	104	155	8	0	25	0	0	0	0	0	0	234	0	0	0	0	132	22	0	16	696	
4:30 PM	121	145	13	0	30	0	0	0	0	0	1	252	0	0	0	0	190	29	0	19	800	
4:45 PM	179	189	11	0	22	0	0	0	0	0	0	279	0	0	0	0	195	21	0	16	912	3247
5:00 PM	120	127	21	0	20	0	0	0	0	0	0	241	0	0	0	0	89	10	0	21	649	3057
<b>5:15 PM</b>	<b>158</b>	<b>161</b>	<b>13</b>	<b>0</b>	<b>23</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>248</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>270</b>	<b>30</b>	<b>1</b>	<b>25</b>	<b>929</b>	<b>3290</b>
5:30 PM	141	166	11	0	17	0	0	0	0	0	0	256	0	0	0	0	209	16	0	16	832	3322
5:45 PM	133	157	12	0	25	0	0	0	0	0	1	225	0	0	0	0	167	31	0	11	762	3172

Peak 15-Min Flowrates	Northbound					Southbound					Eastbound					Westbound					<b>Total</b>	
	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*		
All Vehicles	632	644	52	0	92	0	0	0	0	0	0	992	0	0	0	0	1080	120	4	100	3716	
Heavy Trucks	20	28	0	0	0	0	0	0	0	0	0	28	0	0	0	0	56	0	0	0	132	
Pedestrians			12																4		16	
Bicycles	0	0	0			0	0	0			0	0	0			0	0	0			0	
Railroad																						
Stopped Buses																						

*Comments:*

Type of peak hour being reported: Intersection Peak

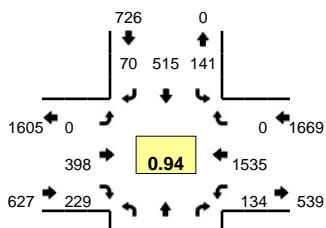
Method for determining peak hour: Total Entering Volume

**LOCATION:** 21st St -- SR 60

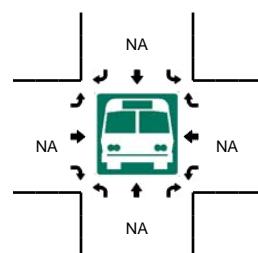
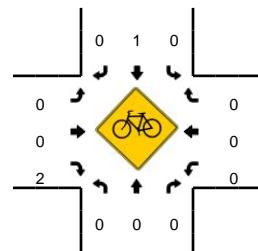
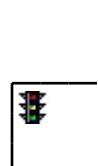
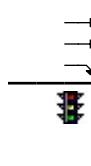
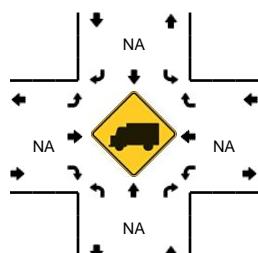
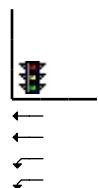
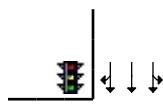
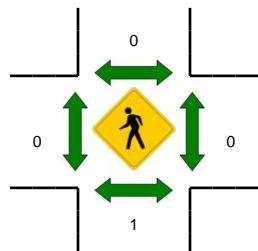
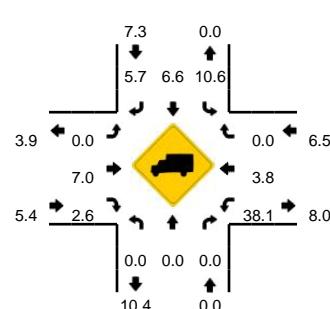
**CITY/STATE:** Tampa, FL

**QC JOB #:** 14664810

**DATE:** Thu, Mar 29 2018



**Peak-Hour: 7:15 AM -- 8:15 AM**  
**Peak 15-Min: 7:15 AM -- 7:30 AM**



R\* = RTOR

15-Min Count Period Beginning At	21st St (Northbound)					21st St (Southbound)					SR 60 (Eastbound)					SR 60 (Westbound)					Total	Hourly Totals
	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*		
7:00 AM	0	0	0	0	0	29	146	16	0	1	0	41	10	0	29	22	364	0	0	0	658	
7:15 AM	0	0	0	0	0	39	168	24	0	3	0	94	25	0	22	36	390	0	0	0	801	
7:30 AM	0	0	0	0	0	34	97	15	0	0	0	111	33	0	50	35	423	0	0	0	798	
7:45 AM	0	0	0	0	0	43	132	12	0	0	0	106	24	0	34	27	384	0	0	0	762	3019
8:00 AM	0	0	0	0	0	25	118	13	0	3	0	87	10	0	31	36	338	0	0	0	661	3022
8:15 AM	0	0	0	0	0	29	114	10	0	2	0	99	20	0	37	26	387	0	0	0	724	2945
8:30 AM	0	0	0	0	0	24	71	8	0	1	0	106	12	0	34	29	352	0	0	0	637	2784
8:45 AM	0	0	0	0	0	36	103	15	0	1	0	87	5	0	33	24	313	0	0	0	617	2639

Peak 15-Min Flowrates	Northbound					Southbound					Eastbound					Westbound					Total	
	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*		
All Vehicles	0	0	0	0	0	156	672	96	0	12	0	376	100	0	88	144	1560	0	0	0	3204	
Heavy Trucks	0	0	0			20	28	4			0	12	4			52	60	0			180	
Pedestrians	0					0					0					0					0	
Bicycles	0	0	0			0	0	0			0	0	0			0	0	0			0	
Railroad																						
Stopped Buses																						

*Comments:*

Report generated on 4/12/2018 6:27 AM

SOURCE: Quality Counts, LLC (<http://www.qualitycounts.net>) 1-877-580-2212

Type of peak hour being reported: Intersection Peak

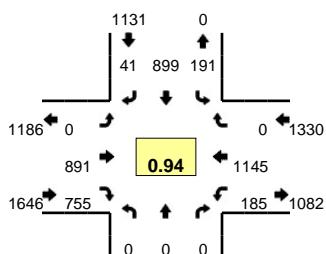
Method for determining peak hour: Total Entering Volume

**LOCATION:** 21st St -- SR 60

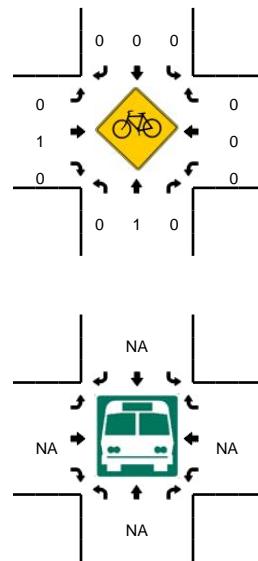
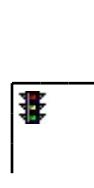
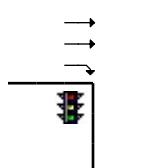
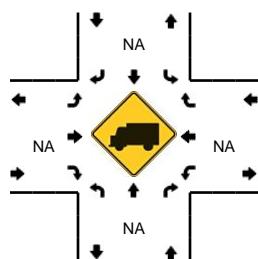
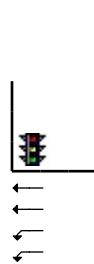
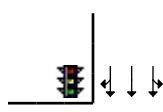
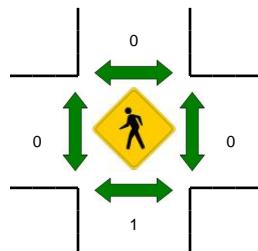
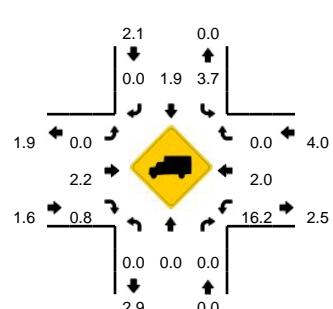
**CITY/STATE:** Tampa, FL

**QC JOB #:** 14664812

**DATE:** Thu, Mar 29 2018



**Peak-Hour: 4:45 PM -- 5:45 PM**  
**Peak 15-Min: 5:15 PM -- 5:30 PM**



R\* = RTOR

15-Min Count Period Beginning At	21st St (Northbound)				21st St (Southbound)				SR 60 (Eastbound)				SR 60 (Westbound)				Total	Hourly Totals	
	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U	Left	Thru	Right	U			
4:00 PM	0	0	0	0	44	233	10	0	2	0	197	77	0	87	45	247	0	0	942
4:15 PM	0	0	0	0	58	239	15	0	1	0	190	91	0	70	51	235	0	0	950
4:30 PM	0	0	0	0	51	211	9	0	0	0	209	124	0	62	51	274	0	0	991
4:45 PM	0	0	0	0	41	200	10	0	0	0	238	98	0	95	35	296	0	0	1013
5:00 PM	0	0	0	0	53	221	12	0	2	0	189	87	0	91	45	275	0	0	975
<b>5:15 PM</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>52</b>	<b>253</b>	<b>4</b>	<b>0</b>	<b>3</b>	<b>0</b>	<b>232</b>	<b>121</b>	<b>0</b>	<b>69</b>	<b>56</b>	<b>307</b>	<b>0</b>	<b>0</b>	<b>1097</b>
5:30 PM	0	0	0	0	45	225	10	0	0	0	232	126	0	68	49	267	0	0	1022
5:45 PM	0	0	0	0	31	234	9	0	0	0	194	103	0	79	50	235	0	0	935
																		4029	

Peak 15-Min Flowrates	Northbound					Southbound					Eastbound					Westbound					Total
	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	Left	Thru	Right	U	R*	
All Vehicles	0	0	0	0	0	208	1012	16	0	12	0	928	484	0	276	224	1228	0	0	0	4388
Heavy Trucks	0	0	0			12	12	0			0	16	8			24	28	0			100
Pedestrians	0																				0
Bicycles	0	0	0			0	0	0			0	1	0			0	0	0			1
Railroad																					
Stopped Buses																					

**Comments:**

# City of Tampa Signal Timing Sheet

Section ID: 408 Computer: M CCU: 28 Drop: 2 Shop ID: 1506

Timing Date: 4/28/2015 Phase Date: 5/5/2015 Controller: ASC3

Intersection: ADAMO / 34TH ST

Phase Numbers	1	2	3	4	5	6	7	8
Direction	EBLT	WB	SBLT	NB	WBLT	EB	NBLT	SB
Minimum Green	5	15	5	10	5	15	5	10
Walk	---	7	---	7	---	7	---	7
Flash Don't Walk	---	26	---	32	---	26	---	32
Vehicle Extension	2.0	3.0	2.0	3.0	2.0	3.0	2.0	3.0
Max. Green I	20	140	25	40	20	140	25	40
Max. Green II	20	150	25	40	20	150	25	40
Yellow Clearance	5.1	5.1	4.5	4.5	5.1	5.1	4.5	4.5
All Red Clearance	2.1	2.0	2.4	3.2	2.1	2.0	2.4	3.2
Phase Recall	---	MAX	---	---	---	MAX	---	---
Detector Memory	ON	---	ON	---	ON	---	ON	---
Ped. Recall	---	ON	---	---	---	ON	---	---
Flash Operation	RED	YEL	RED	RED	RED	YEL	RED	RED

## Special Modes and Times of Operation:

Surveillance Times:

Flash Source: Flash Times:

C = Computer Flash T = Time Clock/Controller

Special Functions:

FDOT SOP: 10 MOD

Backup Protection (Y/N): Y

FDOT FDW (Y/N): Y

Please Implement Within:  1 Week  1 Month

## Comments:


Submitted By: JL Reviewed By: RS Approved By: WJ  
 Date: 1-21-15 Date: 1-21-15 Date: 1/24/15

Signal Timing Implemented:  As sent.  With the following revisions

Date: 23 Jul 15 By: RS

Signal Timing Not Implemented:  Reasons: \_\_\_\_\_

Date: \_\_\_\_\_ By: \_\_\_\_\_

## CITY OF TAMPA COMPUTER PATTERN SHEET

408

# 408 - ADAMO &amp; 34TH ST

ECONOLITE

		MIN	15	5	10	5		
		YEL	5.1	4.5	4.5	5.1		
		RED	2	2.4	3.2	2.1		
		WLK	7		7			
		FDW	26		32			
Structures: 1								
Lead / Lag:		Min - 84	41	12	18	13		
Pat		CYC	OS	EW	NSLT	NS	EWLT	
1	Am	0600 - 0915	190	121	98	25	47	20
2	Am off	0915 - 1115	180	70	88	25	47	20
3	Noon	1115 - 1330	180	70	88	25	47	20
4	Pm off	1330 - 1445	180	70	88	25	47	20
5	Pm	1445 - 1900	190	182	105	20	50	15
6	Evening	1900 - 2200	160	75	83	16	47	14
7	Late	2200 - 0600	160	75	83	16	47	14
8			140	30	68	12	47	13
9	X-town Detour	EB	180	10	105	12	50	13
10			120	62	48	12	47	13
11	Large Band	EB	200	124	128	12	47	13
12	Guavaween	IN	150	53	78	12	47	13
13	Guavaween	OUT	150	53	78	12	47	13
14	Larger band	WB	250	236	178	12	47	13
15	Large band	WB	200	92	128	12	47	13
16	Hurricane		250	123	178	12	47	13

T.B.C. Day Plan 1: M-Th patt 1-7 Day Plan 2: Fri patt 1-7  
 Day Plan 3: S-Su patt 7 and patt 2 @ 6:15 to 22:00

**FDOT SOP#** 1 **Sect. I.D.#** 408

## **City of Tampa - Phasing Diagram**

## **City of Tampa - Phasing Diagram**

Diagram 5/5/2015

5/5/2015

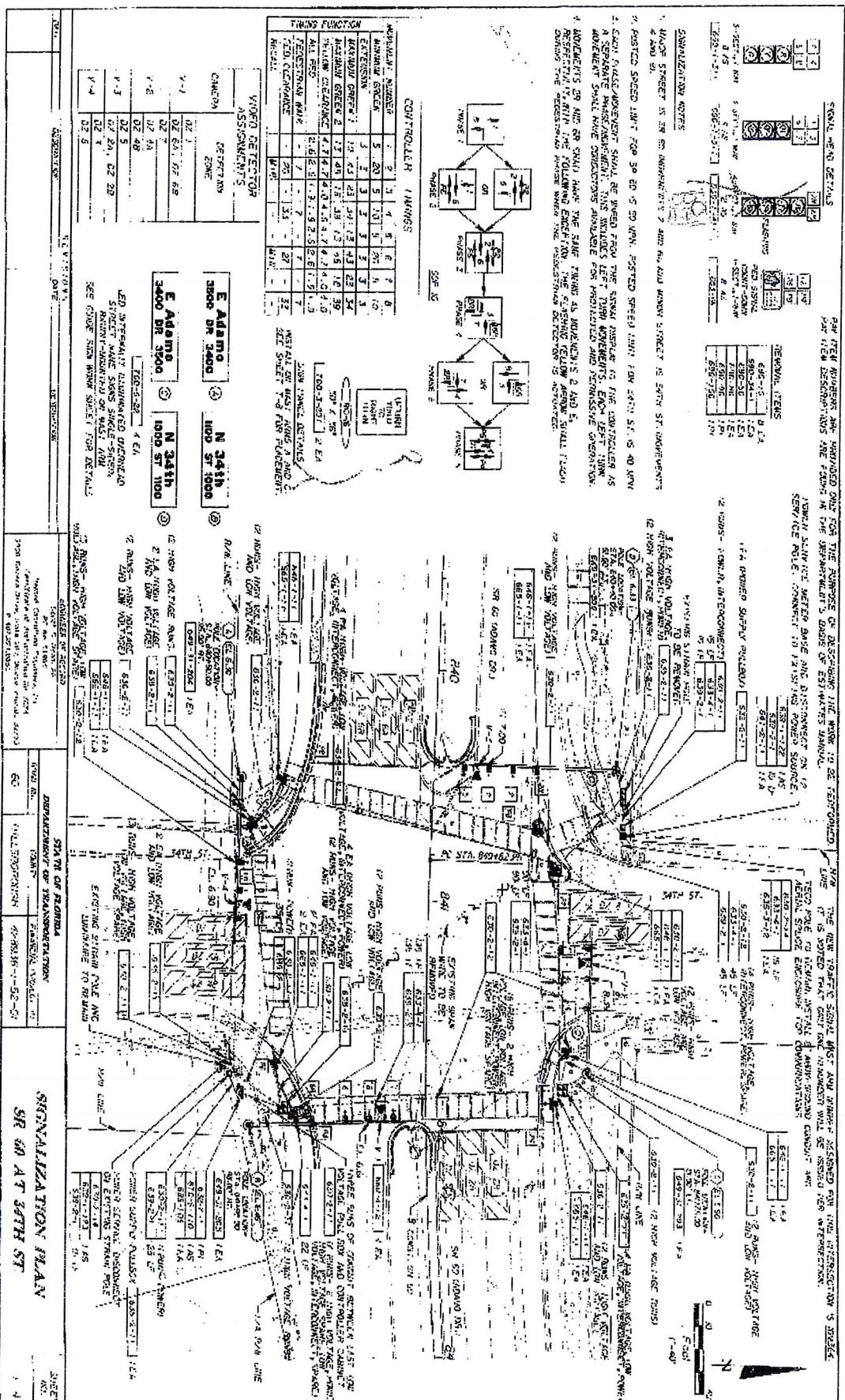
# City of Tampa - Phasing Diagram

FDOT SOP# 1 Sect. I.D.# 408

Phasing Date: 2/15/2001

# Intersection Drawings

Ø Diagram 5/5/2015  
Form Vers. 21012014



NOTICE: THE ATTACHED DRAWING IN THIS SHEET IS THE PROPERTY OF STANCO AND SHOULD NEVER PRICE ABOVE \$1,000.00.

# City of Tampa Signal Timing Sheet

Section ID: 405 Computer: M CCU: 28 Drop: 1 Shop ID: 1821

Timing Date: 1/27/2015 Phase Date: 1/23/2014 Controller: PEEK 3000

Intersection: ADAMO / 21ST ST / 22ND ST

Phase Numbers	2 E/W	3 WBLT	6 N - W	7 S-E
Minimum Green	15	5	10	10
Walk	7	---	7	7
Flash Don't Walk	19	---	23	28
Vehicle Extension	3.0	3.0	3.0	3.0
Max. Green i	35	15	35	35
Max. Green II	50	20	60	50
Yellow Clearance	4.5	4.5	4.0	3.7
All Red Clearance	2.5	2.5	2.9	3.3
Phase Recall	MAX	MAX	MAX	MAX
Detector Memory	---	---	---	---
Ped. Recall	ON	---	---	---
Flash Operation	YEL	RED	---	---

## Special Modes and Times of Operation:

Surveillance Times:

Flash Source: Flash Times:

C = Computer Flash T = Time Clock/Controller

Special Functions:

FDOT SOP: 20 MOD

Backup Protection (Y/N): N

FDOT FDW (Y/N): Y

Please Implement Within : [ ] 1 Week [ ] 1 Month

## Comments:

\*Overlap assignments - OLE (2+3+4+7), OLF (2), OLG (7), OLI (2+3+6), OLJ (2+3), OLK (6), \*

\*OLL (3+6), P2, POLN (6), POLP (7).\*

\*Phasing Sequence Ø2(OLE+OLF+OLI+OLJ+P2+P2A+P6A+P6B), Ø3(OLE+OLI+OLJ+OLL), Ø7(OLE+OLG+POLP), \*

\*Ø6(OLI+OLK+OLL+POLN).\*

\*Actuated Pre-Timed Operation.\*

Submitted By: NM Reviewed By: \_\_\_\_\_ Approved By: \_\_\_\_\_

Date: 1/27/15 Date: \_\_\_\_\_ Date: \_\_\_\_\_

Signal Timing Implemented: [  ] As sent . [ ] With the following revisions

Date: 1/27/15 By: NM

Signal Timing Not Implemented: [ ] Reasons: \_\_\_\_\_

Date: \_\_\_\_\_ By: \_\_\_\_\_

405  
CITY OF TAMPA COMPUTER PATTERN SHEET

405

# 405 - ADAMO & 21ST ST & 22ND ST

PEEK 3000E

Timing Date: 09/09/2016		MIN	15	5	10	10		
MSX: M CCU: 28 Drop: 1		YEL	4.5	4.5	3.7	4		
Structures: 1		RED	2.5	2.5	3.3	2.9		
Lead / Lag:		WLK	7	7	7	7		
		FDW	19	28	23			
		Min - 82	34	13	18	17		
Pat		CYC	OS	EW	WBLT	SBLT		
1	Am	0600 - 0915	190	74	55	20	55	60
2	Am off	0915 - 1115	180	28	55	20	45	60
3	Noon	1115 - 1330	180	28	55	20	45	60
4	Pm off	1330 - 1445	180	28	55	20	45	60
5	Pm	1445 - 1900	190	139	65	15	55	55
6	Evening	1900 - 2200	160	26	50	20	42	48
7	Late	2200 - 0600	160	23	61	20	42	37
8			140	134	35	27	37	41
9	X-town Detour	EB	180	152	90	13	36	41
10			140	134	35	27	37	41
11	Large Band	EB	200	71	110	13	36	41
12	Guavaween	IN	150	128	49	13	46	42
13	Guavaween	OUT	150	128	44	13	36	57
14	Larger band	WB	250	55	40	133	36	41
15	Large band	WB	200	139	40	83	36	41
16	Hurricane		250	63	160	13	36	41

T.B.C. Day Plan 1: M-Th patt 1-7 Day Plan 2: Fri patt 1-7  
Day Plan 3: S-Su patt 7 and patt 2 @ 6:15 to 22:00



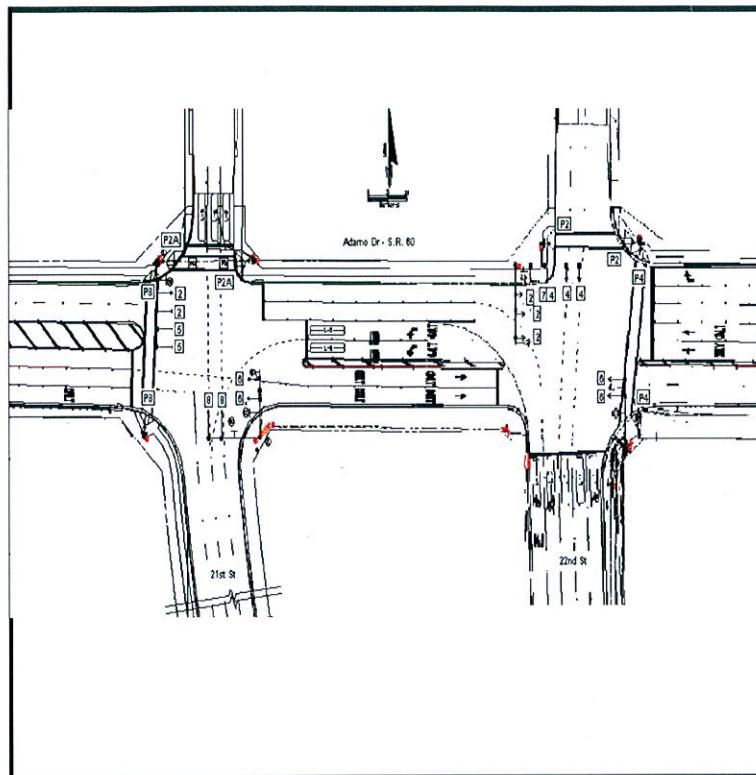
# City of Tampa - Phasing Diagram



DWG 1/26/2011

Vers. 8/27/2010

Pg: 1 of 2



Sect. I.D.#

405

Location:

ADAMO / 21ST ST &amp; 22ND ST

Prepared by GT Reviewed by *[Signature]*

Phasing Date:

Overlaps

1/26/2011

OL8 OL6 OL5

OL7

OL2 OL1

OL7

OL3

1 2 2A

4

6 6A

7

8

Signal Head Numbers

P2A P2 P6 P8

Controller:

Econolite

R Y Y

R

Y Y

R

R

Vehicle Movements

Flashing Operation

&lt; R

Y Y

R

Phase

Interval

Display Sequence

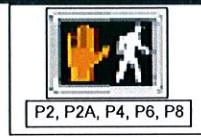
W DW DW

FDW DW DW

DW DW DW



1,7

2, 2A, 4, 6,  
6A, 8

P2, P2A, P4, P6, P8

8 phase controller w/ 6 phases in actuated pre-timed sequential operation. CNA phase is Ø2. Ped heads and buttons on P2, P2A, P6, and P8. Phases are Ø2(OL1+OL2+OL5+OL6+P2+P2A), Ø3(OL1+OL5+OL6+OL8), Ø4(OL1+OL5), Ø5(OL3+OL7+POL4+POL8), Ø6(OL5+OL7+OL8+POL4), Ø7(OL1+OL3+POL8).

FDOT SOP 20 MOD

Signal Head #	1	2	2A	4	6	6A	7	8	P2	P6	P8
Econolite Overlaps	OL8	OL6	OL5	OL7	OL2	OL1	OL7	OL3		POL4	POL8
Load Switch #	LS8	LS6	LS5	LS7	LS2	LS1	LS7	LS3		LS9	LS10
Peek Overlaps	OLL	OLJ	OLI	OLK	OLF	OLE	OLK	OLG		POLN	POLP



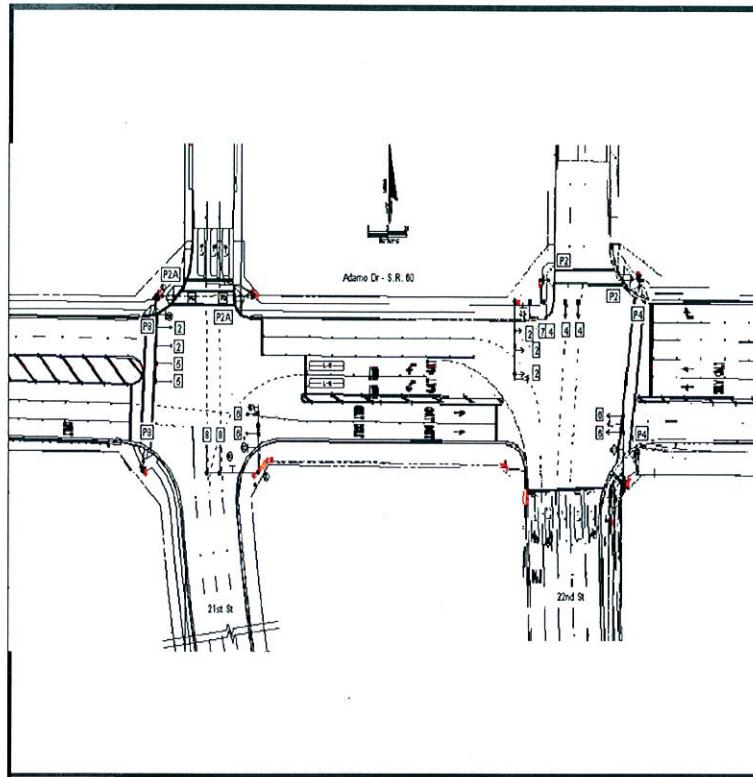
# City of Tampa - Phasing Diagram



DWG 1/26/2011

Vers. 8/27/2010

Pg: 2 of 2



Sect. I.D.# 405

Location:

ADAMO / 21ST ST & 22ND ST

Prepared by GT Reviewed by *[Signature]*

Phasing Date: Overlaps

1/26/2011 Signal Head Numbers

OL8 OL6 OL5

OL7

OL2 OL1

OL7

OL3

OL8

OL3

OL8

OL3

OL8

OL3

OL8

OL3

OL8

Controller: Econolite

OLL OLJ OLI

OLK

OLF OLE

OLK

OLF OLE

OLK

OLF OLE

OLK

OLF OLE

Vehicle Movements

Flashing Operation

← R

Y Y

R

Y Y

R

Y Y

R

Y Y

R

Y Y

R

Y Y

R

Y Y

Phase Interval

Display Sequence

RW ← R R R G R R G G DW W W

Ø5 Clr to Ped ← R R R G R R G G DW FDW FDW

OL3 Clr to ← R R R G R R G Y DW W DW

OL7 Ø6 & Clr to ← R R R G R R G R DW W DW

POL4 Ø7 Clr to ← R R R R R R R G DW DW W

POL8 Clr to ← R R R Y R R Y Y DW DW DW

Ø2 Clr to ← R R R R R R R R DW DW DW DW

POL4 Clr to ← R R R R R R R R DW DW DW DW

OL5 Ø6 Clr to Ped ← G R G G R R G R DW W DW

OL5 Clr to ← Y R Y Y R R Y R DW DW DW

OL7 Ø7 Clr to ← R R R R R R R R DW DW DW DW

OL8 Clr to ← Y R G Y R R Y R DW DW DW DW

& Ø2 Clr to ← R R G R R R R R R DW DW DW DW

POL4 Clr to ← R R R R R R R R DW DW DW DW

OL7 Clr to ← R R R R R R R R DW DW DW DW

POL4 Clr to ← R R R R R R R R DW DW DW DW

OL5 Ø7 Clr to Ped ← R R R R R R R G DW DW W

OL1 Clr to ← R R R R R R G R G DW DW DW

OL3 Ø2 Clr to ← R R R R R R G R R G DW DW DW

POL8 Clr to ← R R R R R R G R R G DW DW DW

OL1 Clr to ← R R R R R R G R R G DW DW DW

OL3 Ø2 Clr to ← R R R R R R G R R G DW DW DW

POL8 Clr to ← R R R R R R G R R G DW DW DW

OL1 Clr to ← R R R R R R G R R G DW DW DW

OL3 Ø2 Clr to ← R R R R R R G R R G DW DW DW

POL8 Clr to ← R R R R R R G R R G DW DW DW

OL1 Clr to ← R R R R R R G R R G DW DW DW

OL3 Ø2 Clr to ← R R R R R R G R R G DW DW DW

POL8 Clr to ← R R R R R R G R R G DW DW DW

OL1 Clr to ← R R R R R R G R R G DW DW DW

OL3 Ø2 Clr to ← R R R R R R G R R G DW DW DW

POL8 Clr to ← R R R R R R G R R G DW DW DW

OL1 Clr to ← R R R R R R G R R G DW DW DW

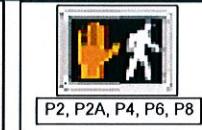
OL3 Ø2 Clr to ← R R R R R R G R R G DW DW DW

POL8 Clr to ← R R R R R R G R R G DW DW DW

OL1 Clr to ← R R R R R R G R R G DW DW DW

OL3 Ø2 Clr to ← R R R R R R G R R G DW DW DW

POL8 Clr to ← R R R R R R G R R G DW DW DW

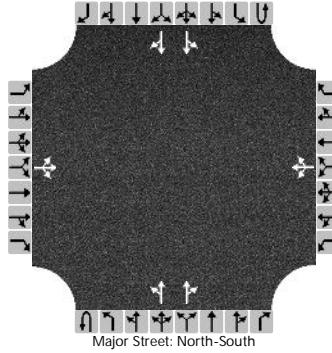


8 phase controller w/ 6 phases in actuated pre-timed sequential operation. CNA phase is Ø2. Ped heads and buttons on P2, P2A, P6, and P8. Phases are Ø2(OL1+OL2+OL5+OL6+P2+P2A), Ø3(OL1+OL5+OL6+OL8), Ø4(OL1+OL5), Ø5(OL3+OL7+POL4+POL8), Ø6(OL5+OL7+OL8+POL4), Ø7(OL1+OL3+POL8).

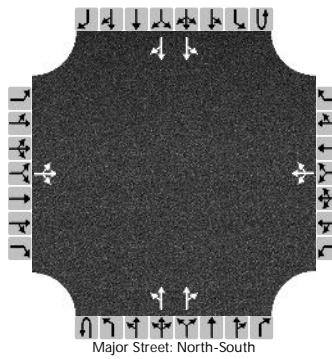
FDOT SOP 20 MOD

Signal Head #	1	2	2A	4	6	6A	7	8	P2	P6	P8
Econolite Overlaps	OL8	OL6	OL5	OL7	OL2	OL1	OL7	OL3		POL4	POL8
Load Switch #	LS8	LS6	LS5	LS7	LS2	LS1	LS7	LS3	LS9	LS10	LS12
Peek Overlaps	OLL	OLJ	OLI	OLK	OLF	OLE	OLK	OLG		POLN	POLP

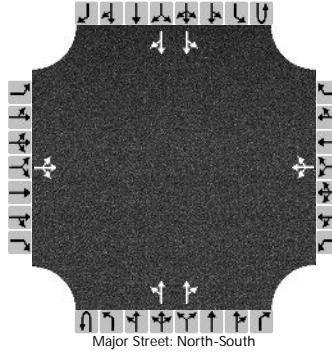
# HCS7 Two-Way Stop-Control Report

General Information				Site Information																																
Analyst	MDS			Intersection			Clark Street and 34th Str																													
Agency/Co.	GFY			Jurisdiction			City of Tampa																													
Date Performed	04/25/2018			East/West Street			Clark Street																													
Analysis Year	2018			North/South Street			34th Street																													
Time Analyzed	AM Peak Hour			Peak Hour Factor			0.79																													
Intersection Orientation	North-South			Analysis Time Period (hrs)			1.00																													
Project Description	Clark Street Distribution Center																																			
Lanes																																				
 Major Street: North-South																																				
Vehicle Volumes and Adjustments																																				
Approach	Eastbound				Westbound				Northbound				Southbound																							
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U																							
Priority		10	11	12		7	8	9	1U	1	2	3	4U																							
Number of Lanes	0	1	0		0	1	0	0	0	0	2	0	0																							
Configuration		LTR				LTR			LT		TR		LT																							
Volume (veh/h)	55	0	13		5	0	13		9	25	34		5																							
Percent Heavy Vehicles (%)	8	0	0		100	0	0		50				100																							
Proportion Time Blocked																																				
Percent Grade (%)	0				0																															
Right Turn Channelized																																				
Median Type   Storage	Left + Thru								1																											
Critical and Follow-up Headways																																				
Base Critical Headway (sec)		7.5	6.5	6.9		7.5	6.5	6.9		4.1			4.1																							
Critical Headway (sec)		7.66	6.50	6.90		9.50	6.50	6.90		5.10			6.10																							
Base Follow-Up Headway (sec)		3.5	4.0	3.3		3.5	4.0	3.3		2.2			2.2																							
Follow-Up Headway (sec)		3.58	4.00	3.30		4.50	4.00	3.30		2.70			3.20																							
Delay, Queue Length, and Level of Service																																				
Flow Rate, v (veh/h)			86				23			11			6																							
Capacity, c (veh/h)			662				828			983			1025																							
v/c Ratio			0.13				0.03			0.01			0.01																							
95% Queue Length, Q <sub>95</sub> (veh)			0.4				0.1			0.0			0.0																							
Control Delay (s/veh)			11.3				9.5			8.7			8.5																							
Level of Service (LOS)			B				A			A			A																							
Approach Delay (s/veh)	11.3				9.5				1.2			0.2																								
Approach LOS	B				A																															

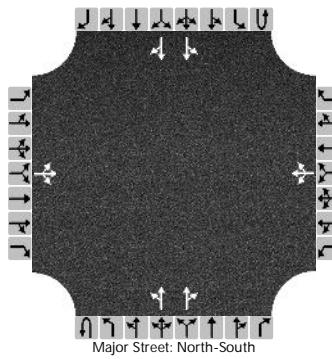
# HCS7 Two-Way Stop-Control Report

General Information				Site Information																																
Analyst	MDS			Intersection			Clark Street and 34th Str																													
Agency/Co.	GFY			Jurisdiction			City of Tampa																													
Date Performed	04/25/2018			East/West Street			Clark Street																													
Analysis Year	2018			North/South Street			34th Street																													
Time Analyzed	PM Peak Hour			Peak Hour Factor			0.76																													
Intersection Orientation	North-South			Analysis Time Period (hrs)			1.00																													
Project Description	Clark Street Distribution Center																																			
Lanes																																				
 Major Street: North-South																																				
Vehicle Volumes and Adjustments																																				
Approach	Eastbound				Westbound				Northbound				Southbound																							
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U																							
Priority		10	11	12		7	8	9	1U	1	2	3	4U																							
Number of Lanes	0	1	0		0	1	0	0	0	0	2	0	0																							
Configuration		LTR				LTR			LT		TR		LT																							
Volume (veh/h)	28	0	0		0	0	12		12	96	0	4	20																							
Percent Heavy Vehicles (%)	14	0	0		0	0	33		33			0																								
Proportion Time Blocked																																				
Percent Grade (%)	0				0																															
Right Turn Channelized																																				
Median Type   Storage	Left + Thru								1																											
Critical and Follow-up Headways																																				
Base Critical Headway (sec)		7.5	6.5	6.9		7.5	6.5	6.9		4.1			4.1																							
Critical Headway (sec)		7.78	6.50	6.90		7.50	6.50	7.56		4.76			4.10																							
Base Follow-Up Headway (sec)		3.5	4.0	3.3		3.5	4.0	3.3		2.2			2.2																							
Follow-Up Headway (sec)		3.64	4.00	3.30		3.50	4.00	3.63		2.53			2.20																							
Delay, Queue Length, and Level of Service																																				
Flow Rate, v (veh/h)		37				16			16			5																								
Capacity, c (veh/h)		711				896			1252			1473																								
v/c Ratio		0.05				0.02			0.01			0.00																								
95% Queue Length, Q <sub>95</sub> (veh)		0.2				0.1			0.0			0.0																								
Control Delay (s/veh)		10.3				9.1			7.9			7.5																								
Level of Service (LOS)		B				A			A			A																								
Approach Delay (s/veh)	10.3				9.1				0.9			0.3																								
Approach LOS	B				A																															

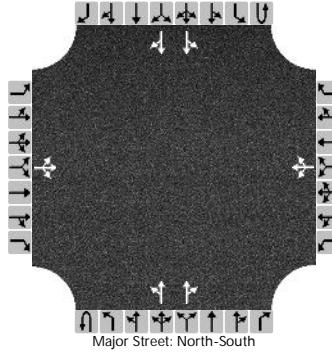
# HCS7 Two-Way Stop-Control Report

General Information				Site Information																																
Analyst	MDS			Intersection			Clark Street and 34th Str																													
Agency/Co.	GFY			Jurisdiction			City of Tampa																													
Date Performed	4/25/2018			East/West Street			Clark Street																													
Analysis Year	2018			North/South Street			34th Street																													
Time Analyzed	AM Peak Hour			Peak Hour Factor			0.79																													
Intersection Orientation	North-South			Analysis Time Period (hrs)			1.00																													
Project Description	Clark Street Distribution Center																																			
Lanes																																				
 Major Street: North-South																																				
Vehicle Volumes and Adjustments																																				
Approach	Eastbound				Westbound				Northbound				Southbound																							
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U																							
Priority		10	11	12		7	8	9	1U	1	2	3	4U																							
Number of Lanes		0	1	0		0	1	0	0	0	2	0	0																							
Configuration		LTR				LTR				LT		TR																								
Volume (veh/h)		55	0	13		5	0	13		9	25	34	5																							
Percent Heavy Vehicles (%)		15	0	0		100	0	33		56			0																							
Proportion Time Blocked																																				
Percent Grade (%)	0				0																															
Right Turn Channelized																																				
Median Type   Storage	Left + Thru								1																											
Critical and Follow-up Headways																																				
Base Critical Headway (sec)		7.5	6.5	6.9		7.5	6.5	6.9		4.1			4.1																							
Critical Headway (sec)		7.80	6.50	6.90		9.50	6.50	7.56		5.22			4.10																							
Base Follow-Up Headway (sec)		3.5	4.0	3.3		3.5	4.0	3.3		2.2			2.2																							
Follow-Up Headway (sec)		3.65	4.00	3.30		4.50	4.00	3.63		2.76			2.20																							
Delay, Queue Length, and Level of Service																																				
Flow Rate, v (veh/h)			86				23			11			6																							
Capacity, c (veh/h)			647				780			954			1537																							
v/c Ratio			0.13				0.03			0.01			0.00																							
95% Queue Length, Q <sub>95</sub> (veh)			0.5				0.1			0.0			0.0																							
Control Delay (s/veh)			11.4				9.8			8.8			7.4																							
Level of Service (LOS)			B				A			A			A																							
Approach Delay (s/veh)	11.4				9.8				1.2			0.2																								
Approach LOS	B				A																															

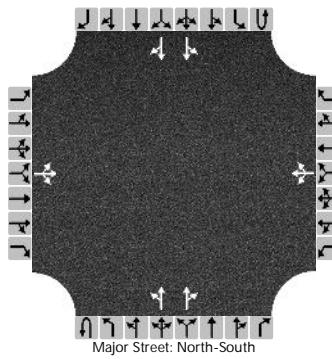
# HCS7 Two-Way Stop-Control Report

General Information				Site Information																																
Analyst	MDS			Intersection			Clark Street and 34th Str																													
Agency/Co.	GFY			Jurisdiction			City of Tampa																													
Date Performed	04/25/2018			East/West Street			Clark Street																													
Analysis Year	2018			North/South Street			34th Street																													
Time Analyzed	PM Peak Hour			Peak Hour Factor			0.76																													
Intersection Orientation	North-South			Analysis Time Period (hrs)			1.00																													
Project Description	Clark Street Distribution Center																																			
Lanes																																				
 Major Street: North-South																																				
Vehicle Volumes and Adjustments																																				
Approach	Eastbound				Westbound				Northbound				Southbound																							
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U																							
Priority		10	11	12		7	8	9	1U	1	2	3	4U																							
Number of Lanes		0	1	0		0	1	0	0	0	2	0	0																							
Configuration		LTR				LTR			LT		TR		LT																							
Volume (veh/h)		30	0	0		0	0	13		13	100	0	5																							
Percent Heavy Vehicles (%)		14	0	0		0	0	33		33			0																							
Proportion Time Blocked																																				
Percent Grade (%)	0				0																															
Right Turn Channelized																																				
Median Type   Storage	Left + Thru								1																											
Critical and Follow-up Headways																																				
Base Critical Headway (sec)		7.5	6.5	6.9		7.5	6.5	6.9		4.1			4.1																							
Critical Headway (sec)		7.78	6.50	6.90		7.50	6.50	7.56		4.76			4.10																							
Base Follow-Up Headway (sec)		3.5	4.0	3.3		3.5	4.0	3.3		2.2			2.2																							
Follow-Up Headway (sec)		3.64	4.00	3.30		3.50	4.00	3.63		2.53			2.20																							
Delay, Queue Length, and Level of Service																																				
Flow Rate, v (veh/h)			39				17			17			7																							
Capacity, c (veh/h)			699				893			1244			1466																							
v/c Ratio			0.06				0.02			0.01			0.00																							
95% Queue Length, Q <sub>95</sub> (veh)			0.2				0.1			0.0			0.0																							
Control Delay (s/veh)			10.5				9.1			7.9			7.5																							
Level of Service (LOS)			B				A			A			A																							
Approach Delay (s/veh)	10.5				9.1				1.0			0.3																								
Approach LOS			B				A																													

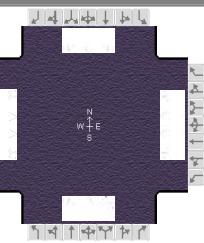
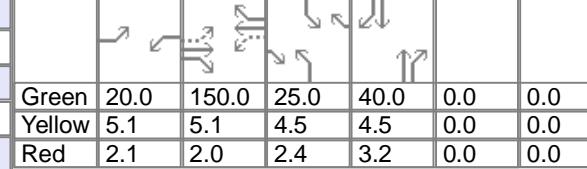
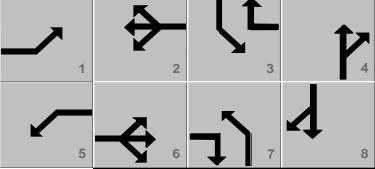
# HCS7 Two-Way Stop-Control Report

General Information				Site Information																																
Analyst	MDS			Intersection			Clark Street and 34th Str																													
Agency/Co.	GFY			Jurisdiction			City of Tampa																													
Date Performed	4/25/2018			East/West Street			Clark Street																													
Analysis Year	2018			North/South Street			34th Street																													
Time Analyzed	AM Peak Hour			Peak Hour Factor			0.79																													
Intersection Orientation	North-South			Analysis Time Period (hrs)			1.00																													
Project Description	Clark Street Distribution Center																																			
Lanes																																				
 Major Street: North-South																																				
Vehicle Volumes and Adjustments																																				
Approach	Eastbound				Westbound				Northbound				Southbound																							
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U																							
Priority		10	11	12		7	8	9	1U	1	2	3	4U																							
Number of Lanes	0	1	0		0	1	0	0	0	0	2	0	0																							
Configuration		LTR				LTR			LT		TR		LT																							
Volume (veh/h)	59	0	13		5	0	13		9	27	34		5																							
Percent Heavy Vehicles (%)	15	0	0		100	0	33		56				0																							
Proportion Time Blocked																																				
Percent Grade (%)	0				0																															
Right Turn Channelized																																				
Median Type   Storage	Left + Thru						1																													
Critical and Follow-up Headways																																				
Base Critical Headway (sec)		7.5	6.5	6.9		7.5	6.5	6.9		4.1			4.1																							
Critical Headway (sec)		7.80	6.50	6.90		9.50	6.50	7.56		5.22			4.10																							
Base Follow-Up Headway (sec)		3.5	4.0	3.3		3.5	4.0	3.3		2.2			2.2																							
Follow-Up Headway (sec)		3.65	4.00	3.30		4.50	4.00	3.63		2.76			2.20																							
Delay, Queue Length, and Level of Service																																				
Flow Rate, v (veh/h)			91				23			11			6																							
Capacity, c (veh/h)			636				778			936			1534																							
v/c Ratio			0.14				0.03			0.01			0.00																							
95% Queue Length, Q <sub>95</sub> (veh)			0.5				0.1			0.0			0.0																							
Control Delay (s/veh)			11.6				9.8			8.9			7.4																							
Level of Service (LOS)			B				A			A			A																							
Approach Delay (s/veh)	11.6				9.8				1.2			0.2																								
Approach LOS	B				A																															

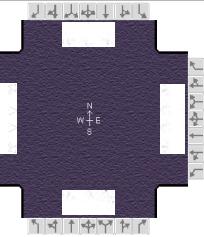
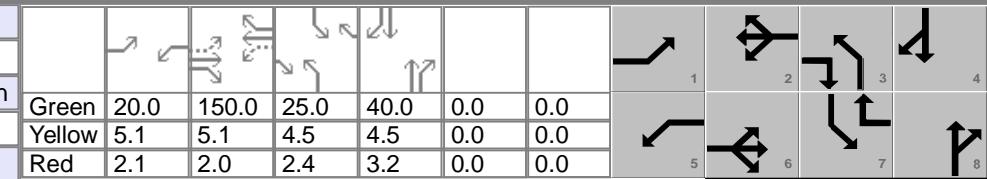
# HCS7 Two-Way Stop-Control Report

General Information				Site Information																																
Analyst	MDS			Intersection			Clark Street and 34th Str																													
Agency/Co.	GFY			Jurisdiction			City of Tampa																													
Date Performed	04/25/2018			East/West Street			Clark Street																													
Analysis Year	2018			North/South Street			34th Street																													
Time Analyzed	PM Peak Hour			Peak Hour Factor			0.76																													
Intersection Orientation	North-South			Analysis Time Period (hrs)			1.00																													
Project Description	Clark Street Distribution Center																																			
Lanes																																				
 Major Street: North-South																																				
Vehicle Volumes and Adjustments																																				
Approach	Eastbound				Westbound				Northbound				Southbound																							
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U																							
Priority		10	11	12		7	8	9	1U	1	2	3	4U																							
Number of Lanes		0	1	0		0	1	0	0	0	2	0	0																							
Configuration		LTR				LTR				LT		TR																								
Volume (veh/h)		40	0	0		0	0	13		13	105	0	5																							
Percent Heavy Vehicles (%)		38	0	0		0	33	38		38			0																							
Proportion Time Blocked																																				
Percent Grade (%)	0				0																															
Right Turn Channelized																																				
Median Type   Storage	Left + Thru								1																											
Critical and Follow-up Headways																																				
Base Critical Headway (sec)		7.5	6.5	6.9		7.5	6.5	6.9		4.1			4.1																							
Critical Headway (sec)		8.26	6.50	6.90		7.50	7.16	7.66		4.86			4.10																							
Base Follow-Up Headway (sec)		3.5	4.0	3.3		3.5	4.0	3.3		2.2			2.2																							
Follow-Up Headway (sec)		3.88	4.00	3.30		3.50	4.33	3.68		2.58			2.20																							
Delay, Queue Length, and Level of Service																																				
Flow Rate, v (veh/h)			53				17			17			7																							
Capacity, c (veh/h)			640				875			1205			1458																							
v/c Ratio			0.08				0.02			0.01			0.00																							
95% Queue Length, Q <sub>95</sub> (veh)			0.3				0.1			0.0			0.0																							
Control Delay (s/veh)			11.1				9.2			8.0			7.5																							
Level of Service (LOS)			B				A			A			A																							
Approach Delay (s/veh)	11.1				9.2				0.9			0.3																								
Approach LOS	B				A																															

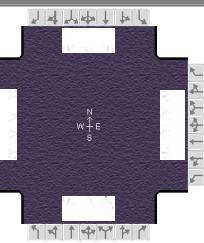
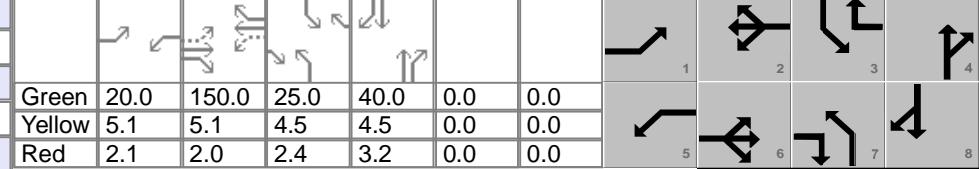
# HCS7 Signalized Intersection Results Summary

General Information						Intersection Information								
Agency	GFY			Duration, h			1.00							
Analyst	MDS		Analysis Date	Apr 16, 2018		Area Type		Other						
Jurisdiction	City of Tampa		Time Period	AM Peak		PHF		1.00						
Urban Street	State Road 60		Analysis Year	2018		Analysis Period		1> 7:00						
Intersection	34th Street and State R...			File Name		34th-SR60-EX-AM.xus								
Project Description	Clark Street Distribution Center													
Demand Information				EB		WB		NB		SB				
Approach Movement				L	T	R	L	T	R	L				
Demand ( v ), veh/h				92	604	64	60	1100	148	28				
										T				
										R				
Signal Information														
Cycle, s	263.9	Reference Phase	2											
Offset, s	0	Reference Point	Begin	Green	20.0	150.0	25.0	40.0	0.0	0.0				
Uncoordinated	Yes	Simult. Gap E/W	On	Yellow	5.1	5.1	4.5	4.5	0.0	0.0				
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.1	2.0	2.4	3.2	0.0	0.0				
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT			
Assigned Phase				1	6	5	2	7	4	3	8			
Case Number				1.1	3.0	1.1	3.0	2.0	4.0	2.0	3.0			
Phase Duration, s				27.2	157.1	27.2	157.1	31.9	47.7	31.9	47.7			
Change Period, ( Y+R <sub>c</sub> ), s				7.2	7.1	7.2	7.1	6.9	7.7	6.9	7.7			
Max Allow Headway ( MAH ), s				3.1	4.0	3.1	4.0	3.2	4.3	3.2	4.3			
Queue Clearance Time ( g <sub>s</sub> ), s				7.0	26.4	6.1	56.7	13.8	16.0	18.7	10.3			
Green Extension Time ( g <sub>e</sub> ), s				0.1	11.6	0.1	11.6	0.0	0.7	0.1	0.7			
Phase Call Probability				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00			
Max Out Probability				0.00	0.00	0.00	0.00	0.00	0.00	0.02	0.00			
Movement Group Results				EB		WB		NB		SB				
Approach Movement				L	T	R	L	T	R	L	T	R		
Assigned Movement				1	6	16	5	2	12	7	4	14		
Adjusted Flow Rate ( v ), veh/h				92	604	64	60	1100	112	28	20	80		
Adjusted Saturation Flow Rate ( s ), veh/h/ln				1810	1710	1133	1428	1696	1553	596	1604	1359		
Queue Service Time ( g <sub>s</sub> ), s				5.0	24.4	5.3	4.1	54.7	6.9	11.8	2.8	14.0		
Cycle Queue Clearance Time ( g <sub>c</sub> ), s				5.0	24.4	5.3	4.1	54.7	6.9	11.8	2.8	14.0		
Green Ratio ( g/C )				0.64	0.57	0.66	0.64	0.57	0.66	0.09	0.15	0.15		
Capacity ( c ), veh/h				349	1944	751	442	1928	1032	56	243	206		
Volume-to-Capacity Ratio ( X )				0.264	0.311	0.085	0.136	0.571	0.109	0.496	0.082	0.388		
Back of Queue ( Q ), ft/ln ( 50 th percentile)				55.4	276.7	48.8	43.5	621.7	67.5	80.4	35.1	127.8		
Back of Queue ( Q ), veh/ln ( 50 th percentile)				2.2	10.5	1.5	1.4	23.4	2.6	1.9	1.2	5.1		
Queue Storage Ratio ( RQ ) ( 50 th percentile)				0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Uniform Delay ( d <sub>1</sub> ), s/veh				25.0	29.9	15.9	18.9	36.4	16.1	113.5	96.2	100.9		
Incremental Delay ( d <sub>2</sub> ), s/veh				0.1	0.4	0.2	0.1	1.2	0.2	2.5	0.1	1.2		
Initial Queue Delay ( d <sub>3</sub> ), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
Control Delay ( d ), s/veh				25.1	30.3	16.1	18.9	37.6	16.4	116.0	96.3	102.1		
Level of Service (LOS)				C	C	B	B	D	B	F	F	F		
Approach Delay, s/veh / LOS				28.5	C	34.9	C	104.2	F	112.9	F			
Intersection Delay, s/veh / LOS				43.6				D						
Multimodal Results				EB		WB		NB		SB				
Pedestrian LOS Score / LOS				2.14	B	2.40	B	2.54	C	2.49	B			
Bicycle LOS Score / LOS				1.11	A	1.54	B	0.59	A	0.84	A			

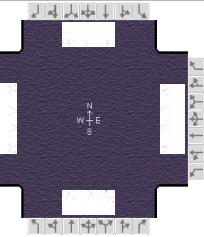
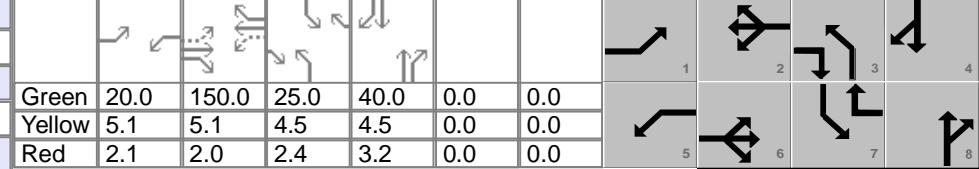
# HCS7 Signalized Intersection Results Summary

General Information						Intersection Information						
Agency	GFY			Duration, h		1.00						
Analyst	MDS		Analysis Date	Apr 13, 2018		Area Type		Other				
Jurisdiction	City of Tampa		Time Period	PM Peak		PHF		1.00				
Urban Street	State Road 60		Analysis Year	2018		Analysis Period		1> 7:00				
Intersection	34th Street and State R...		File Name	34th-SR60-EX-PM.xus								
Project Description	Clark Street Distribution Center											
Demand Information				EB		WB		NB		SB		
Approach Movement				L	T	R	L	T	R	L	T	R
Demand ( v ), veh/h				100	1576	20	56	1204	156	32	36	20
Signal Information												
Cycle, s	263.9	Reference Phase	2									
Offset, s	0	Reference Point	Begin	Green	20.0	150.0	25.0	40.0	0.0	0.0		
Uncoordinated	Yes	Simult. Gap E/W	On	Yellow	5.1	5.1	4.5	4.5	0.0	0.0		
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.1	2.0	2.4	3.2	0.0	0.0		
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	
Assigned Phase				1	6	5	2	3	8	7	4	
Case Number				1.1	3.0	1.1	3.0	2.0	4.0	2.0	3.0	
Phase Duration, s				27.2	157.1	27.2	157.1	31.9	47.7	31.9	47.7	
Change Period, ( Y+R <sub>c</sub> ), s				7.2	7.1	7.2	7.1	6.9	7.7	6.9	7.7	
Max Allow Headway ( MAH ), s				3.1	4.0	3.1	4.0	3.2	4.1	3.2	4.1	
Queue Clearance Time ( g <sub>s</sub> ), s				7.9	93.7	5.6	61.6	9.3	5.0	26.2	6.7	
Green Extension Time ( g <sub>e</sub> ), s				0.1	26.5	0.0	30.5	0.0	0.3	0.0	0.3	
Phase Call Probability				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Max Out Probability				0.00	0.24	0.00	0.10	0.00	0.00	1.00	0.00	
Movement Group Results				EB		WB		NB		SB		
Approach Movement				L	T	R	L	T	R	L	T	R
Assigned Movement				1	6	16	5	2	12	3	8	18
Adjusted Flow Rate ( v ), veh/h				100	1576	20	56	1204	120	32	20	20
Adjusted Saturation Flow Rate ( s ), veh/h/ln				1681	1766	1723	1513	1752	1667	1083	1574	1525
Queue Service Time ( g <sub>s</sub> ), s				5.9	91.7	1.0	3.6	59.6	6.9	7.3	2.9	3.0
Cycle Queue Clearance Time ( g <sub>c</sub> ), s				5.9	91.7	1.0	3.6	59.6	6.9	7.3	2.9	3.0
Green Ratio ( g/C )				0.64	0.57	0.66	0.64	0.57	0.66	0.09	0.15	0.15
Capacity ( c ), veh/h				303	2008	1146	201	1992	1105	103	239	231
Volume-to-Capacity Ratio ( X )				0.330	0.785	0.017	0.279	0.604	0.109	0.312	0.084	0.086
Back of Queue ( Q ), ft/ln ( 50 th percentile)				64.8	1053.	11	47	679	70.5	73.9	35.7	30.3
Back of Queue ( Q ), veh/ln ( 50 th percentile)				2.4	41.1	0.4	1.6	26.3	2.8	2.1	1.2	1.2
Queue Storage Ratio ( RQ ) ( 50 th percentile)				0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Uniform Delay ( d <sub>1</sub> ), s/veh				27.1	44.4	15.1	38.2	37.4	16.1	111.4	96.2	96.2
Incremental Delay ( d <sub>2</sub> ), s/veh				0.2	3.2	0.0	0.3	1.4	0.2	0.6	0.1	0.2
Initial Queue Delay ( d <sub>3</sub> ), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay ( d ), s/veh				27.3	47.6	15.2	38.5	38.8	16.3	112.1	96.4	96.4
Level of Service (LOS)				C	D	B	D	D	B	F	F	F
Approach Delay, s/veh / LOS				46.0		D	36.9		D	103.3		F
Intersection Delay, s/veh / LOS				53.3						D		
Multimodal Results				EB		WB		NB		SB		
Pedestrian LOS Score / LOS				2.14	B	2.40	B	2.54	C	2.49	B	
Bicycle LOS Score / LOS				1.89	B	1.63	B	0.55	A	0.85	A	

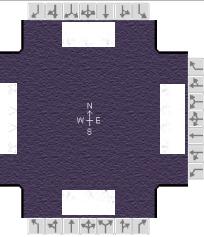
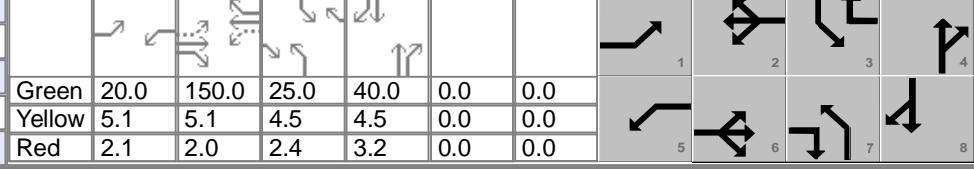
# HCS7 Signalized Intersection Results Summary

General Information						Intersection Information							
Agency	GFY			Duration, h		1.00							
Analyst	MDS		Analysis Date	Apr 16, 2018		Area Type		Other					
Jurisdiction	City of Tampa		Time Period	AM Peak		PHF		1.00					
Urban Street	State Road 60		Analysis Year	2018		Analysis Period		1> 7:00					
Intersection	34th Street and State R...			File Name	34th-SR60-FB-AM.xus								
Project Description	Clark Street Distribution Center												
Demand Information			EB		WB		NB		SB				
Approach Movement			L	T	R	L	T	R	L				
Demand ( v ), veh/h			96	629	67	63	1145	154	30				
									T				
									R				
Signal Information													
Cycle, s	263.9	Reference Phase	2										
Offset, s	0	Reference Point	Begin	Green	20.0	150.0	25.0	40.0					
Uncoordinated	Yes	Simult. Gap E/W	On	Yellow	5.1	5.1	4.5	4.5					
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.1	2.0	2.4	3.2					
Timer Results					EBL	EBT	WBL	WBT	NBL				
Assigned Phase					1	6	5	2	7				
Case Number					1.1	3.0	1.1	3.0	2.0				
Phase Duration, s					27.2	157.1	27.2	157.1	31.9				
Change Period, ( Y+R <sub>c</sub> ), s					7.2	7.1	7.2	7.1	6.9				
Max Allow Headway ( MAH ), s					3.1	4.0	3.1	4.0	3.2				
Queue Clearance Time ( g <sub>s</sub> ), s					7.3	27.7	6.3	60.0	14.7				
Green Extension Time ( g <sub>e</sub> ), s					0.1	12.3	0.1	12.3	0.0				
Phase Call Probability					1.00	1.00	1.00	1.00	1.00				
Max Out Probability					0.00	0.00	0.00	0.00	0.06				
Movement Group Results					EB	WB		NB		SB			
Approach Movement					L	T	R	L	T	R			
Assigned Movement					1	6	16	5	2	12			
Adjusted Flow Rate ( v ), veh/h					96	629	42	63	1145	120			
Adjusted Saturation Flow Rate ( s ), veh/h/ln					1810	1710	1133	1428	1696	1553			
Queue Service Time ( g <sub>s</sub> ), s					5.3	25.7	3.4	4.3	58.0	7.4			
Cycle Queue Clearance Time ( g <sub>c</sub> ), s					5.3	25.7	3.4	4.3	58.0	7.4			
Green Ratio ( g/C )					0.64	0.57	0.66	0.64	0.57	0.66			
Capacity ( c ), veh/h					335	1944	751	432	1928	1032			
Volume-to-Capacity Ratio ( X )					0.287	0.324	0.056	0.146	0.594	0.116			
Back of Queue ( Q ), ft/ln ( 50 th percentile)					58	291	31.3	45.7	659.9	72.8			
Back of Queue ( Q ), veh/ln ( 50 th percentile)					2.3	11.0	1.0	1.5	24.8	2.8			
Queue Storage Ratio ( RQ ) ( 50 th percentile)					0.00	0.00	0.00	0.00	0.00	0.00			
Uniform Delay ( d <sub>1</sub> ), s/veh					26.1	30.1	15.6	19.1	37.1	16.2			
Incremental Delay ( d <sub>2</sub> ), s/veh					0.2	0.4	0.1	0.1	1.4	0.2			
Initial Queue Delay ( d <sub>3</sub> ), s/veh					0.0	0.0	0.0	0.0	0.0	0.0			
Control Delay ( d ), s/veh					26.3	30.6	15.7	19.1	38.5	16.5			
Level of Service (LOS)					C	C	B	B	D	B			
Approach Delay, s/veh / LOS					29.2	C	35.6	D	106.4	F			
Intersection Delay, s/veh / LOS					42.2				D				
Multimodal Results					EB	WB		NB		SB			
Pedestrian LOS Score / LOS					2.24	B	2.45	B	2.54	C			
Bicycle LOS Score / LOS					1.12	A	1.58	B	0.54	A			

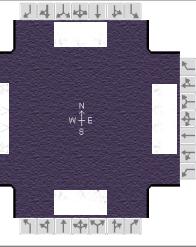
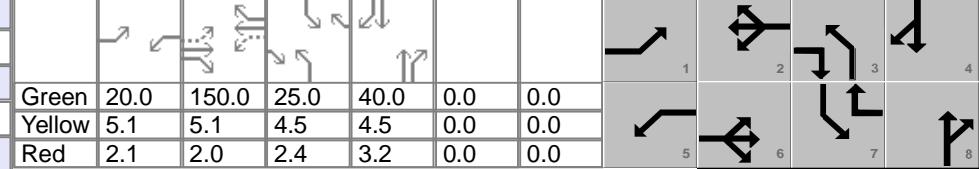
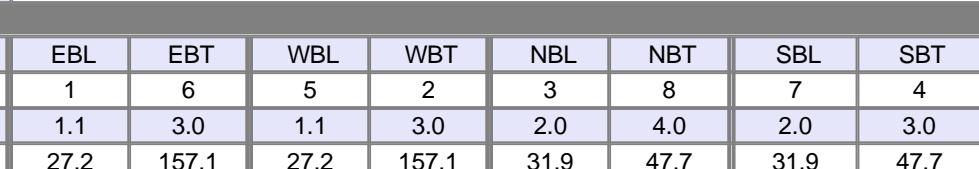
# HCS7 Signalized Intersection Results Summary

General Information						Intersection Information					
Agency	GFY			Duration, h			1.00				
Analyst	MDS		Analysis Date	Apr 26, 2018		Area Type		Other			
Jurisdiction	City of Tampa		Time Period	PM Peak		PHF		1.00			
Urban Street	State Road 60		Analysis Year	2018		Analysis Period		1> 7:00			
Intersection	34th Street and State R...			File Name		34th-SR60-FB-PM.xus					
Project Description	Clark Street Distribution Center										
Demand Information			EB		WB		NB		SB		
Approach Movement			L	T	R	L	T	R	L		
Demand ( v ), veh/h			105	1640	21	59	1253	163	34		
									R		
									38		
									105		
Signal Information											
Cycle, s	263.9	Reference Phase	2								
Offset, s	0	Reference Point	Begin	Green	20.0	150.0	25.0	40.0	0.0		
Uncoordinated	Yes	Simult. Gap E/W	On	Yellow	5.1	5.1	4.5	4.5	0.0		
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.1	2.0	2.4	3.2	0.0		
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT		
Assigned Phase				1	6	5	2	3	8		
Case Number				1.1	3.0	1.1	3.0	2.0	4.0		
Phase Duration, s				27.2	157.1	27.2	157.1	31.9	47.7		
Change Period, ( Y+R <sub>c</sub> ), s				7.2	7.1	7.2	7.1	6.9	7.7		
Max Allow Headway ( MAH ), s				3.1	4.0	3.1	4.0	3.2	4.2		
Queue Clearance Time ( g <sub>s</sub> ), s				8.3	100.7	5.8	65.4	9.6	5.1		
Green Extension Time ( g <sub>e</sub> ), s				0.1	26.9	0.1	33.3	0.0	0.3		
Phase Call Probability				1.00	1.00	1.00	1.00	1.00	1.00		
Max Out Probability				0.00	0.34	0.00	0.15	0.00	0.00		
Movement Group Results				EB		WB		NB			
Approach Movement				L	T	R	L	T	R		
Assigned Movement				1	6	16	5	2	12		
Adjusted Flow Rate ( v ), veh/h				105	1640	21	59	1253	125		
Adjusted Saturation Flow Rate ( s ), veh/h/ln				1681	1766	1723	1513	1752	1667		
Queue Service Time ( g <sub>s</sub> ), s				6.3	98.7	1.1	3.8	63.4	7.2		
Cycle Queue Clearance Time ( g <sub>c</sub> ), s				6.3	98.7	1.1	3.8	63.4	7.2		
Green Ratio ( g/C )				0.64	0.57	0.66	0.64	0.57	0.66		
Capacity ( c ), veh/h				290	2008	1146	190	1992	1105		
Volume-to-Capacity Ratio ( X )				0.362	0.817	0.018	0.310	0.629	0.113		
Back of Queue ( Q ), ft/ln ( 50 th percentile)				68.3	1128.1	11.4	55.4	716.2	72.5		
Back of Queue ( Q ), veh/ln ( 50 th percentile)				2.6	44.1	0.5	1.9	27.8	2.9		
Queue Storage Ratio ( RQ ) ( 50 th percentile)				0.00	0.00	0.00	0.00	0.00	0.00		
Uniform Delay ( d <sub>1</sub> ), s/veh				28.6	45.9	15.2	42.2	38.3	16.2		
Incremental Delay ( d <sub>2</sub> ), s/veh				0.3	2.8	0.0	0.3	0.6	0.0		
Initial Queue Delay ( d <sub>3</sub> ), s/veh				0.0	0.0	0.0	0.0	0.0	0.0		
Control Delay ( d ), s/veh				28.9	48.7	15.2	42.6	38.9	16.2		
Level of Service (LOS)				C	D	B	D	B	F		
Approach Delay, s/veh / LOS				47.1		D	37.1	D	103.5		
Intersection Delay, s/veh / LOS				57.1				E			
Multimodal Results				EB		WB		NB			
Pedestrian LOS Score / LOS				2.14	B	2.41	B	2.55	C		
Bicycle LOS Score / LOS				1.94	B	1.67	B	0.55	A		

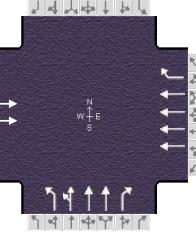
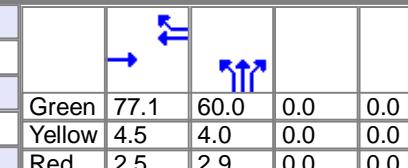
# HCS7 Signalized Intersection Results Summary

General Information						Intersection Information					
Agency	GFY			Duration, h			1.00				
Analyst	MDS		Analysis Date	Apr 16, 2018		Area Type		Other			
Jurisdiction	City of Tampa		Time Period	AM Peak		PHF		1.00			
Urban Street	State Road 60		Analysis Year	2018		Analysis Period		1> 7:00			
Intersection	34th Street and State R...			File Name		34th-SR60-FBP-AM.xus					
Project Description	Clark Street Distribution Center										
Demand Information			EB		WB		NB		SB		
Approach Movement			L	T	R	L	T	R	L		
Demand ( v ), veh/h			96	629	74	70	1145	154	33		
									T		
									R		
Signal Information											
Cycle, s	263.9	Reference Phase	2								
Offset, s	0	Reference Point	Begin	Green	20.0	150.0	25.0	40.0			
Uncoordinated	Yes	Simult. Gap E/W	On	Yellow	5.1	5.1	4.5	4.5			
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.1	2.0	2.4	3.2			
Timer Results					EBL	EBT	WBL	WBT	NBL		
Assigned Phase					1	6	5	2	7		
Case Number					1.1	3.0	1.1	3.0	2.0		
Phase Duration, s					27.2	157.1	27.2	157.1	31.9		
Change Period, ( Y+R <sub>c</sub> ), s					7.2	7.1	7.2	7.1	6.9		
Max Allow Headway ( MAH ), s					3.1	4.0	3.1	4.0	3.2		
Queue Clearance Time ( g <sub>s</sub> ), s					7.3	27.7	7.2	60.0	16.8		
Green Extension Time ( g <sub>e</sub> ), s					0.1	12.4	0.1	12.4	0.0		
Phase Call Probability					1.00	1.00	1.00	1.00	1.00		
Max Out Probability					0.00	0.00	0.00	0.00	0.06		
Movement Group Results					EB	WB		NB		SB	
Approach Movement					L	T	R	L	T	R	
Assigned Movement					1	6	16	5	2	12	
Adjusted Flow Rate ( v ), veh/h					96	629	49	70	1145	120	
Adjusted Saturation Flow Rate ( s ), veh/h/ln					1810	1710	1045	1330	1696	1553	
Queue Service Time ( g <sub>s</sub> ), s					5.3	25.7	4.4	5.2	58.0	7.4	
Cycle Queue Clearance Time ( g <sub>c</sub> ), s					5.3	25.7	4.4	5.2	58.0	7.4	
Green Ratio ( g/C )					0.64	0.57	0.66	0.64	0.57	0.66	
Capacity ( c ), veh/h					335	1944	693	404	1928	1032	
Volume-to-Capacity Ratio ( X )					0.287	0.324	0.071	0.173	0.594	0.116	
Back of Queue ( Q ), ft/ln ( 50 th percentile)					58	291	38.6	53.7	659.9	72.8	
Back of Queue ( Q ), veh/ln ( 50 th percentile)					2.3	11.0	1.1	1.7	24.8	2.8	
Queue Storage Ratio ( RQ ) ( 50 th percentile)					0.00	0.00	0.00	0.00	0.00	0.00	
Uniform Delay ( d <sub>1</sub> ), s/veh					26.1	30.1	15.7	19.3	37.1	16.2	
Incremental Delay ( d <sub>2</sub> ), s/veh					0.2	0.4	0.2	0.1	1.4	0.2	
Initial Queue Delay ( d <sub>3</sub> ), s/veh					0.0	0.0	0.0	0.0	0.0	0.0	
Control Delay ( d ), s/veh					26.3	30.6	15.9	19.4	38.5	16.5	
Level of Service (LOS)					C	C	B	B	D	B	
Approach Delay, s/veh / LOS					29.1		35.5		D	111.6	
Intersection Delay, s/veh / LOS					42.4				D		
Multimodal Results					EB	WB		NB		SB	
Pedestrian LOS Score / LOS					2.24	B	2.45	B	2.54	C	
Bicycle LOS Score / LOS					1.13	A	1.59	B	0.55	A	

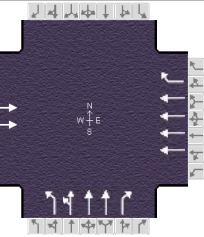
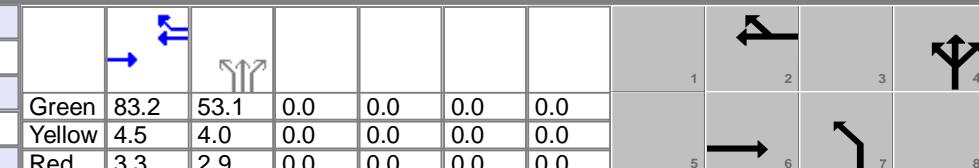
# HCS7 Signalized Intersection Results Summary

General Information						Intersection Information					
Agency		GFY				Duration, h		1.00			
Analyst		MDS		Analysis Date		Apr 26, 2018		Area Type	Other		
Jurisdiction		City of Tampa		Time Period		PM Peak		PHF	1.00		
Urban Street		State Road 60		Analysis Year		2018		Analysis Period	1> 7:00		
Intersection		34th Street and State R...			File Name		34th-SR60-FBP-PM.xus				
Project Description		Clark Street Distribution Center									
Demand Information				EB		WB		NB		SB	
Approach Movement				L	T	R	L	T	R	L	
Demand ( v ), veh/h				105	1640	25	62	1253	163	42	
										T	
										R	
Signal Information											
Cycle, s	263.9	Reference Phase			2						
Offset, s	0	Reference Point			Begin	Green	20.0	150.0	25.0	40.0	
Uncoordinated	Yes	Simult. Gap E/W			On	Yellow	5.1	5.1	4.5	4.5	
Force Mode	Fixed	Simult. Gap N/S			On	Red	2.1	2.0	2.4	3.2	
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase				1	6	5	2	3	8	7	4
Case Number				1.1	3.0	1.1	3.0	2.0	4.0	2.0	3.0
Phase Duration, s				27.2	157.1	27.2	157.1	31.9	47.7	31.9	47.7
Change Period, ( Y+R <sub>c</sub> ), s				7.2	7.1	7.2	7.1	6.9	7.7	6.9	7.7
Max Allow Headway ( MAH ), s				3.1	4.0	3.1	4.0	3.2	4.2	3.2	4.2
Queue Clearance Time ( g <sub>s</sub> ), s				8.3	100.7	6.2	65.4	12.9	5.7	27.0	7.1
Green Extension Time ( g <sub>e</sub> ), s				0.1	26.9	0.1	33.3	0.0	0.3	0.0	0.3
Phase Call Probability				1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Max Out Probability				0.00	0.34	0.00	0.15	0.00	0.00	1.00	0.00
Movement Group Results				EB		WB		NB		SB	
Approach Movement				L	T	R	L	T	R	L	
Assigned Movement				1	6	16	5	2	12	3	8
Adjusted Flow Rate ( v ), veh/h				105	1640	25	62	1253	125	42	25
Adjusted Saturation Flow Rate ( s ), veh/h/ln				1681	1766	1508	1443	1752	1667	963	1544
Queue Service Time ( g <sub>s</sub> ), s				6.3	98.7	1.5	4.2	63.4	7.2	10.9	3.6
Cycle Queue Clearance Time ( g <sub>c</sub> ), s				6.3	98.7	1.5	4.2	63.4	7.2	10.9	3.6
Green Ratio ( g/C )				0.64	0.57	0.66	0.64	0.57	0.66	0.09	0.15
Capacity ( c ), veh/h				290	2008	1003	183	1992	1105	91	234
Volume-to-Capacity Ratio ( X )				0.362	0.817	0.025	0.339	0.629	0.113	0.461	0.105
Back of Queue ( Q ), ft/ln ( 50 th percentile)				68.3	1128.1	15.4	62.2	716.2	72.5	104.4	44.5
Back of Queue ( Q ), veh/ln ( 50 th percentile)				2.6	44.1	0.5	2.1	27.8	2.9	2.8	1.5
Queue Storage Ratio ( RQ ) ( 50 th percentile)				0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Uniform Delay ( d <sub>1</sub> ), s/veh				28.6	45.9	15.2	43.1	38.3	16.2	113.1	96.5
Incremental Delay ( d <sub>2</sub> ), s/veh				0.3	2.8	0.0	0.4	0.6	0.0	1.4	0.2
Initial Queue Delay ( d <sub>3</sub> ), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay ( d ), s/veh				28.9	48.7	15.2	43.5	38.9	16.2	114.4	96.7
Level of Service (LOS)				C	D	B	D	D	B	F	F
Approach Delay, s/veh / LOS				47.0		D	37.1		D	104.9	F
Intersection Delay, s/veh / LOS				57.8						E	
Multimodal Results				EB		WB		NB		SB	
Pedestrian LOS Score / LOS				2.14		B	2.41		B	2.55	C
Bicycle LOS Score / LOS				1.95		B	1.68		B	0.56	A

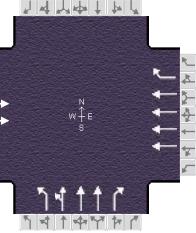
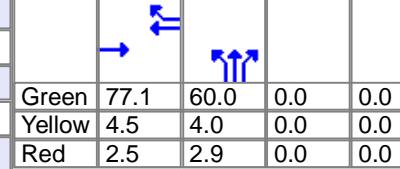
# HCS7 Signalized Intersection Results Summary

General Information						Intersection Information								
Agency		GFY			Duration, h									
Analyst		MDS		Analysis Date		1/29/2018								
Jurisdiction		City of Tampa		Time Period		AM								
Urban Street		State Road 60		Analysis Year		2018								
Intersection		22nd Street and State R...			File Name		22nd-21st-SR60-EX-AM.xus							
Project Description						Clark Street Distribution Center								
Demand Information				EB		WB		NB		SB				
Approach Movement				L	T	R	L	T	R	L	T			
Demand ( v ), veh/h				624			960	216	724	960	88			
Signal Information														
Cycle, s	151.0	Reference Phase	2											
Offset, s	1	Reference Point	End	Green	77.1	60.0	0.0	0.0	0.0	0.0				
Uncoordinated	No	Simult. Gap E/W	Off	Yellow	4.5	4.0	0.0	0.0	0.0	0.0				
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.5	2.9	0.0	0.0	0.0	0.0				
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT			
Assigned Phase					6		2		4					
Case Number					8.0		7.0		9.0					
Phase Duration, s					84.1		84.1		66.9					
Change Period, ( Y+R_c ), s					7.0		7.0		6.9					
Max Allow Headway ( MAH ), s					0.0		0.0		3.1					
Queue Clearance Time ( g_s ), s									62.0					
Green Extension Time ( g_e ), s					0.0		0.0		0.0					
Phase Call Probability									1.00					
Max Out Probability									1.00					
Movement Group Results				EB		WB		NB		SB				
Approach Movement				L	T	R	L	T	R	L	T			
Assigned Movement				6			2	12	7	4	14			
Adjusted Flow Rate ( v ), veh/h				532			960	104	724	960	80			
Adjusted Saturation Flow Rate ( s ), veh/h/ln				1809			1725	1609	1810	1900	1605			
Queue Service Time ( g_s ), s				6.7			11.9	5.1	60.0	18.4	4.8			
Cycle Queue Clearance Time ( g_c ), s				6.7			11.9	5.1	60.0	18.4	4.8			
Green Ratio ( g_C )				0.51			0.51	0.51	0.40	0.40	0.40			
Capacity ( c ), veh/h				1847			3524	822	719	2265	638			
Volume-to-Capacity Ratio ( X )				0.288			0.272	0.127	1.007	0.424	0.125			
Back of Queue ( Q ), ft/ln ( 50 th percentile)				62.2			122.5	50.2	1030	216.2	48.4			
Back of Queue ( Q ), veh/ln ( 50 th percentile)				2.5			4.9	2.0	41.2	8.6	1.9			
Queue Storage Ratio ( RQ ) ( 50 th percentile)				0.00			0.00	0.00	0.00	0.00	0.00			
Uniform Delay ( d_1 ), s/veh				9.5			21.0	19.3	45.5	33.0	28.9			
Incremental Delay ( d_2 ), s/veh				0.4			0.2	0.3	73.9	0.6	0.4			
Initial Queue Delay ( d_3 ), s/veh				0.0			0.0	0.0	0.0	0.0	0.0			
Control Delay ( d ), s/veh				9.8			21.2	19.7	119.4	33.6	29.3			
Level of Service (LOS)				A			C	B	F	C	C			
Approach Delay, s/veh / LOS				9.8	A	21.0	C		68.6	E	0.0			
Intersection Delay, s/veh / LOS						44.2				D				
Multimodal Results				EB		WB		NB		SB				
Pedestrian LOS Score / LOS				2.28	B	1.91	B	2.79	C	2.49	B			
Bicycle LOS Score / LOS				1.00	A	0.93	A	1.46	A					

# HCS7 Signalized Intersection Results Summary

General Information						Intersection Information						
Agency	GFY			Duration, h			1.00					
Analyst	MDS		Analysis Date	Apr 19, 2018		Area Type			Other			
Jurisdiction	City of Tampa		Time Period	PM		PHF			1.00			
Urban Street	State Road 60		Analysis Year	2018		Analysis Period			1> 7:00			
Intersection	22nd Street and State R...			File Name		22nd-21st-SR60-EX-PM.xus						
Project Description	Clark Street Distribution Center											
Demand Information				EB		WB		NB		SB		
Approach Movement				L	T	R	L	T	R	L	T	
Demand ( v ), veh/h				992			1080	220	632	644	144	
Signal Information												
Cycle, s	151.0	Reference Phase	2									
Offset, s	92	Reference Point	End	Green	83.2	53.1	0.0	0.0	0.0	0.0		
Uncoordinated	No	Simult. Gap E/W	Off	Yellow	4.5	4.0	0.0	0.0	0.0	0.0		
Force Mode	Fixed	Simult. Gap N/S	On	Red	3.3	2.9	0.0	0.0	0.0	0.0		
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	
Assigned Phase						6		2		4		
Case Number						8.0		7.0		9.0		
Phase Duration, s						91.0		91.0		60.0		
Change Period, ( Y+R <sub>c</sub> ), s						7.8		7.8		6.9		
Max Allow Headway ( MAH ), s						0.0		0.0		3.1		
Queue Clearance Time ( g <sub>s</sub> ), s										55.1		
Green Extension Time ( g <sub>e</sub> ), s						0.0		0.0		0.0		
Phase Call Probability										1.00		
Max Out Probability										1.00		
Movement Group Results				EB		WB		NB		SB		
Approach Movement				L	T	R	L	T	R	L	T	
Assigned Movement					6			2	12	7	4	
Adjusted Flow Rate ( v ), veh/h				1136			1080	120	632	644	52	
Adjusted Saturation Flow Rate ( s ), veh/h/ln				1766			1658	1610	1767	1841	1610	
Queue Service Time ( g <sub>s</sub> ), s				26.8			13.2	5.5	53.1	12.9	3.3	
Cycle Queue Clearance Time ( g <sub>c</sub> ), s				26.8			13.2	5.5	53.1	12.9	3.3	
Green Ratio ( g/C )				0.55			0.55	0.55	0.35	0.35	0.35	
Capacity ( c ), veh/h				1947			3654	887	621	1942	566	
Volume-to-Capacity Ratio ( X )				0.584			0.296	0.135	1.017	0.332	0.092	
Back of Queue ( Q ), ft/ln ( 50 th percentile)				254			133.2	52.9	987.2	151.1	32.4	
Back of Queue ( Q ), veh/ln ( 50 th percentile)				9.9			5.1	2.1	38.6	5.9	1.3	
Queue Storage Ratio ( RQ ) ( 50 th percentile)				0.00			0.00	0.00	0.00	0.00	0.00	
Uniform Delay ( d <sub>1</sub> ), s/veh				16.0			18.2	16.4	49.0	35.9	32.8	
Incremental Delay ( d <sub>2</sub> ), s/veh				0.9			0.2	0.3	89.7	0.0	0.0	
Initial Queue Delay ( d <sub>3</sub> ), s/veh				0.0			0.0	0.0	0.0	0.0	0.0	
Control Delay ( d ), s/veh				16.9			18.4	16.8	138.7	36.0	32.8	
Level of Service (LOS)				B			B	B	F	D	C	
Approach Delay, s/veh / LOS				16.9	B		18.2	B	84.7	F	0.0	
Intersection Delay, s/veh / LOS						41.9				D		
Multimodal Results				EB		WB		NB		SB		
Pedestrian LOS Score / LOS				2.40	B		1.91	B	2.77	C	2.49	
Bicycle LOS Score / LOS				1.31	A		0.98	A	1.22	A	B	

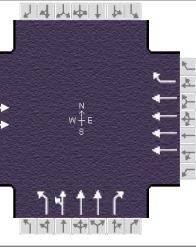
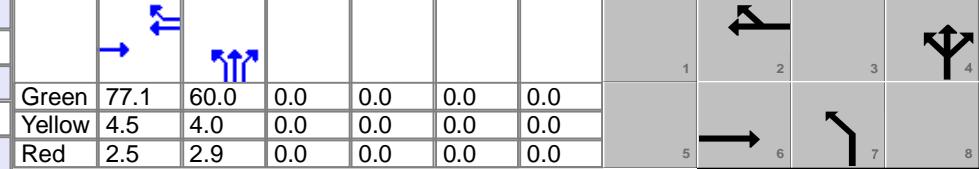
# HCS7 Signalized Intersection Results Summary

General Information						Intersection Information						
Agency	GFY			Duration, h			1.00					
Analyst	MDS		Analysis Date	3/29/2018		Area Type			Other			
Jurisdiction	City of Tampa		Time Period	AM		PHF			1.00			
Urban Street	State Road 60		Analysis Year	2018		Analysis Period			1 > 7:00			
Intersection	22nd Street and State R...			File Name		22nd-21st-SR60-FB-AM.xus						
Project Description	Clark Street Distribution Center											
Demand Information				EB		WB		NB		SB		
Approach Movement				L	T	R	L	T	R	L	T	R
Demand ( v ), veh/h				650			999	225	754	999	92	
Signal Information					1	2	3	4				
Cycle, s	151.0	Reference Phase	2									
Offset, s	1	Reference Point	End		Green	77.1	60.0	0.0	0.0	0.0	0.0	
Uncoordinated	No	Simult. Gap E/W	Off		Yellow	4.5	4.0	0.0	0.0	0.0	0.0	
Force Mode	Fixed	Simult. Gap N/S	On		Red	2.5	2.9	0.0	0.0	0.0	0.0	
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	
Assigned Phase					6		2		4			
Case Number					8.0		7.0		9.0			
Phase Duration, s					84.1		84.1		66.9			
Change Period, ( Y+R <sub>c</sub> ), s					7.0		7.0		6.9			
Max Allow Headway ( MAH ), s					0.0		0.0		3.1			
Queue Clearance Time ( g <sub>s</sub> ), s									62.0			
Green Extension Time ( g <sub>e</sub> ), s					0.0		0.0		0.0			
Phase Call Probability									1.00			
Max Out Probability									1.00			
Movement Group Results				EB		WB		NB		SB		
Approach Movement				L	T	R	L	T	R	L	T	R
Assigned Movement				6			2	12	7	4	14	
Adjusted Flow Rate ( v ), veh/h				555			999	108	754	999	83	
Adjusted Saturation Flow Rate ( s ), veh/h/ln				1696			1523	1522	1753	1870	1384	
Queue Service Time ( g <sub>s</sub> ), s				7.7			14.5	5.6	60.0	19.7	5.8	
Cycle Queue Clearance Time ( g <sub>c</sub> ), s				7.7			14.5	5.6	60.0	19.7	5.8	
Green Ratio ( g/C )				0.51			0.51	0.51	0.40	0.40	0.40	
Capacity ( c ), veh/h				1732			3111	777	697	2230	550	
Volume-to-Capacity Ratio ( X )				0.320			0.321	0.139	1.082	0.448	0.151	
Back of Queue ( Q ), ft/ln ( 50 th percentile)				70.1			147.1	55.5	1543.	231.3	58.5	
Back of Queue ( Q ), veh/ln ( 50 th percentile)				2.6			5.3	2.1	59.8	9.1	2.0	
Queue Storage Ratio ( RQ ) ( 50 th percentile)				0.00			0.00	0.00	0.00	0.00	0.00	
Uniform Delay ( d <sub>1</sub> ), s/veh				9.6			21.6	19.5	45.5	33.4	29.2	
Incremental Delay ( d <sub>2</sub> ), s/veh				0.5			0.3	0.4	176.8	0.7	0.6	
Initial Queue Delay ( d <sub>3</sub> ), s/veh				0.0			0.0	0.0	0.0	0.0	0.0	
Control Delay ( d ), s/veh				10.1			21.9	19.8	222.3	34.0	29.8	
Level of Service (LOS)				B			C	B	F	C	C	
Approach Delay, s/veh / LOS				10.1	B		21.7	C	111.2	F		0.0
Intersection Delay, s/veh / LOS							66.8			E		
Multimodal Results				EB		WB		NB		SB		
Pedestrian LOS Score / LOS				2.29	B		1.91	B	2.80	C	2.49	B
Bicycle LOS Score / LOS				1.02	A		0.94	A	1.50	A		

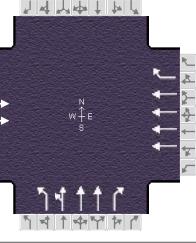
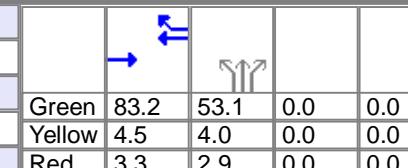
# HCS7 Signalized Intersection Results Summary

General Information						Intersection Information						
Agency	GFY			Duration, h			1.00					
Analyst	MDS		Analysis Date	Apr 19, 2018		Area Type			Other			
Jurisdiction	City of Tampa		Time Period	PM		PHF			1.00			
Urban Street	State Road 60		Analysis Year	2018		Analysis Period			1 > 7:00			
Intersection	22nd Street and State R...			File Name		22nd-21st-SR60-FB-PM.xus						
Project Description	Clark Street Distribution Center											
Demand Information				EB		WB		NB		SB		
Approach Movement				L	T	R	L	T	R	L	T	
Demand ( v ), veh/h				1033			1124	229	658	671	150	
Signal Information												
Cycle, s	151.0	Reference Phase	2									
Offset, s	92	Reference Point	End	Green	83.2	53.1	0.0	0.0	0.0	0.0		
Uncoordinated	No	Simult. Gap E/W	Off	Yellow	4.5	4.0	0.0	0.0	0.0	0.0		
Force Mode	Fixed	Simult. Gap N/S	On	Red	3.3	2.9	0.0	0.0	0.0	0.0		
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT	
Assigned Phase					6		2		4			
Case Number					8.0		7.0		9.0			
Phase Duration, s					91.0		91.0		60.0			
Change Period, ( Y+R <sub>c</sub> ), s					7.8		7.8		6.9			
Max Allow Headway ( MAH ), s					0.0		0.0		3.1			
Queue Clearance Time ( g <sub>s</sub> ), s									55.1			
Green Extension Time ( g <sub>e</sub> ), s					0.0		0.0		0.0			
Phase Call Probability									1.00			
Max Out Probability									1.00			
Movement Group Results				EB		WB		NB		SB		
Approach Movement				L	T	R	L	T	R	L	T	
Assigned Movement				6			2	12	7	4	14	
Adjusted Flow Rate ( v ), veh/h				1183			1124	124	658	671	54	
Adjusted Saturation Flow Rate ( s ), veh/h/ln				1766			1658	1610	1767	1841	1610	
Queue Service Time ( g <sub>s</sub> ), s				28.6			13.8	5.7	53.1	13.5	3.4	
Cycle Queue Clearance Time ( g <sub>c</sub> ), s				28.6			13.8	5.7	53.1	13.5	3.4	
Green Ratio ( g/C )				0.55			0.55	0.55	0.35	0.35	0.35	
Capacity ( c ), veh/h				1947			3654	887	621	1942	566	
Volume-to-Capacity Ratio ( X )				0.608			0.308	0.140	1.059	0.346	0.095	
Back of Queue ( Q ), ft/ln ( 50 th percentile)				270.5			139.6	54.8	1228	158.2	33.7	
Back of Queue ( Q ), veh/ln ( 50 th percentile)				10.6			5.4	2.2	48.0	6.1	1.3	
Queue Storage Ratio ( RQ ) ( 50 th percentile)				0.00			0.00	0.00	0.00	0.00	0.00	
Uniform Delay ( d <sub>1</sub> ), s/veh				16.3			18.3	16.5	49.0	36.1	32.8	
Incremental Delay ( d <sub>2</sub> ), s/veh				0.9			0.2	0.3	144.2	0.0	0.0	
Initial Queue Delay ( d <sub>3</sub> ), s/veh				0.0			0.0	0.0	0.0	0.0	0.0	
Control Delay ( d ), s/veh				17.2			18.5	16.8	193.1	36.2	32.9	
Level of Service (LOS)				B			B	B	F	D	C	
Approach Delay, s/veh / LOS				17.2	B		18.4	B	110.7	F	0.0	
Intersection Delay, s/veh / LOS						51.5				D		
Multimodal Results				EB		WB		NB		SB		
Pedestrian LOS Score / LOS				2.40	B	1.91	B	2.78	C	2.49	B	
Bicycle LOS Score / LOS				1.34	A	1.00	A	1.25	A			

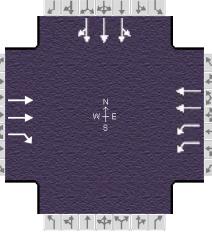
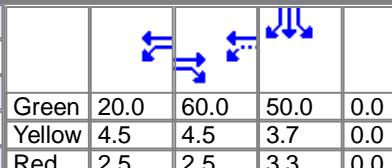
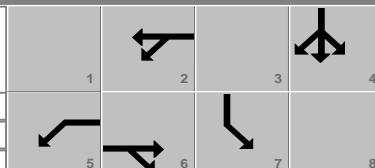
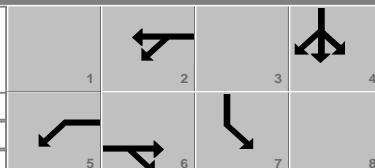
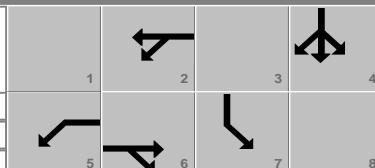
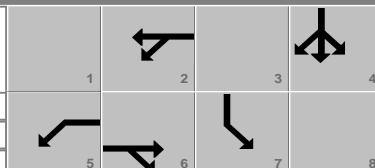
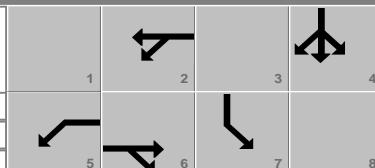
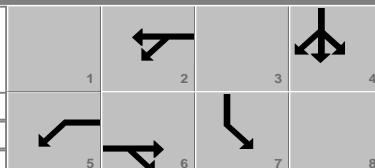
# HCS7 Signalized Intersection Results Summary

General Information						Intersection Information					
Agency	GFY			Duration, h			1.00				
Analyst	MDS		Analysis Date	3/29/2018		Area Type		Other			
Jurisdiction	City of Tampa		Time Period	AM		PHF		1.00			
Urban Street	State Road 60		Analysis Year	2018		Analysis Period		1> 7:00			
Intersection	22nd Street and State R...			File Name		22nd-21st-SR60-FBP-AM.xus					
Project Description	Clark Street Distribution Center										
Demand Information				EB		WB		NB		SB	
Approach Movement				L	T	R	L	T	R	L	
Demand ( v ), veh/h				650			1001	226	754	999	
									99		
Signal Information											
Cycle, s	151.0	Reference Phase	2								
Offset, s	1	Reference Point	End	Green	77.1	60.0	0.0	0.0	0.0		
Uncoordinated	No	Simult. Gap E/W	Off	Yellow	4.5	4.0	0.0	0.0	0.0		
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.5	2.9	0.0	0.0	0.0		
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase					6			2		4	
Case Number					8.0			7.0		9.0	
Phase Duration, s					84.1			84.1		66.9	
Change Period, ( Y+R <sub>c</sub> ), s					7.0			7.0		6.9	
Max Allow Headway ( MAH ), s					0.0			0.0		3.1	
Queue Clearance Time ( g <sub>s</sub> ), s										62.0	
Green Extension Time ( g <sub>e</sub> ), s					0.0			0.0		0.0	
Phase Call Probability									1.00		
Max Out Probability									1.00		
Movement Group Results				EB		WB		NB		SB	
Approach Movement				L	T	R	L	T	R	L	
Assigned Movement				6			2	12	7	4	14
Adjusted Flow Rate ( v ), veh/h				555			1001	109	754	999	90
Adjusted Saturation Flow Rate ( s ), veh/h/ln				1696			1523	1510	1739	1870	1309
Queue Service Time ( g <sub>s</sub> ), s				7.7			14.5	5.8	60.0	19.7	6.7
Cycle Queue Clearance Time ( g <sub>c</sub> ), s				7.7			14.5	5.8	60.0	19.7	6.7
Green Ratio ( g/C )				0.51			0.51	0.51	0.40	0.40	0.40
Capacity ( c ), veh/h				1732			3111	771	691	2230	520
Volume-to-Capacity Ratio ( X )				0.320			0.322	0.141	1.091	0.448	0.173
Back of Queue ( Q ), ft/ln ( 50 th percentile)				70.1			147.6	56.6	1613.	231.3	66.9
					8						
Back of Queue ( Q ), veh/ln ( 50 th percentile)				2.6			5.3	2.1	62.1	9.1	2.2
Queue Storage Ratio ( RQ ) ( 50 th percentile)				0.00			0.00	0.00	0.00	0.00	0.00
Uniform Delay ( d <sub>1</sub> ), s/veh				9.6			21.6	19.5	45.5	33.4	29.4
Incremental Delay ( d <sub>2</sub> ), s/veh				0.5			0.3	0.4	191.0	0.7	0.7
Initial Queue Delay ( d <sub>3</sub> ), s/veh				0.0			0.0	0.0	0.0	0.0	0.0
Control Delay ( d ), s/veh				10.1			21.9	19.9	236.5	34.0	30.2
Level of Service (LOS)				B			C	B	F	C	C
Approach Delay, s/veh / LOS				10.1	B		21.7	C	116.7	F	0.0
Intersection Delay, s/veh / LOS							69.8			E	
Multimodal Results				EB		WB		NB		SB	
Pedestrian LOS Score / LOS				2.29	B		1.91	B	2.80	C	2.49
Bicycle LOS Score / LOS				1.02	A		0.95	A	1.50	B	

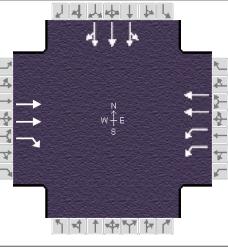
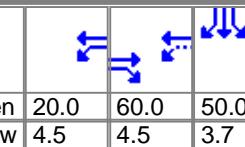
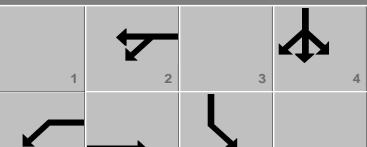
# HCS7 Signalized Intersection Results Summary

General Information						Intersection Information													
Agency	GFY			Duration, h															
Analyst	MDS		Analysis Date	Apr 19, 2018		Area Type													
Jurisdiction	City of Tampa		Time Period	PM		PHF													
Urban Street	State Road 60		Analysis Year	2018		Analysis Period													
Intersection	22nd Street and State R...			File Name		22nd-21st-SR60-FBP-PM.xus													
Project Description	Clark Street Distribution Center																		
Demand Information				EB		WB		NB		SB									
Approach Movement				L	T	R	L	T	R	L	T								
Demand ( v ), veh/h				1033			1124	229	658	671	150								
Signal Information																			
Cycle, s	151.0	Reference Phase	2						1	2									
Offset, s	92	Reference Point	End	Green	83.2	53.1	0.0	0.0	0.0	3	4								
Uncoordinated	No	Simult. Gap E/W	Off	Yellow	4.5	4.0	0.0	0.0	0.0	5	6								
Force Mode	Fixed	Simult. Gap N/S	On	Red	3.3	2.9	0.0	0.0	0.0	7	8								
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT								
Assigned Phase					6		2		4										
Case Number					8.0		7.0		9.0										
Phase Duration, s					91.0		91.0		60.0										
Change Period, ( Y+R <sub>c</sub> ), s					7.8		7.8		6.9										
Max Allow Headway ( MAH ), s					0.0		0.0		3.1										
Queue Clearance Time ( g <sub>s</sub> ), s									55.1										
Green Extension Time ( g <sub>e</sub> ), s					0.0		0.0		0.0										
Phase Call Probability									1.00										
Max Out Probability									1.00										
Movement Group Results				EB		WB		NB		SB									
Approach Movement				L	T	R	L	T	R	L	T								
Assigned Movement				6			2	12	7	4	14								
Adjusted Flow Rate ( v ), veh/h				1183			1124	124	658	671	54								
Adjusted Saturation Flow Rate ( s ), veh/h/ln				1766			1658	1610	1767	1841	1610								
Queue Service Time ( g <sub>s</sub> ), s				28.6			13.8	5.7	53.1	13.5	3.4								
Cycle Queue Clearance Time ( g <sub>c</sub> ), s				28.6			13.8	5.7	53.1	13.5	3.4								
Green Ratio ( g/C )				0.55			0.55	0.55	0.35	0.35	0.35								
Capacity ( c ), veh/h				1947			3654	887	621	1942	566								
Volume-to-Capacity Ratio ( X )				0.608			0.308	0.140	1.059	0.346	0.095								
Back of Queue ( Q ), ft/ln ( 50 th percentile)				270.5			139.6	54.8	1228	158.2	33.7								
Back of Queue ( Q ), veh/ln ( 50 th percentile)				10.6			5.4	2.2	48.0	6.1	1.3								
Queue Storage Ratio ( RQ ) ( 50 th percentile)				0.00			0.00	0.00	0.00	0.00	0.00								
Uniform Delay ( d <sub>1</sub> ), s/veh				16.3			18.3	16.5	49.0	36.1	32.8								
Incremental Delay ( d <sub>2</sub> ), s/veh				0.9			0.2	0.3	144.2	0.0	0.0								
Initial Queue Delay ( d <sub>3</sub> ), s/veh				0.0			0.0	0.0	0.0	0.0	0.0								
Control Delay ( d ), s/veh				17.2			18.5	16.8	193.1	36.2	32.9								
Level of Service (LOS)				B			B	B	F	D	C								
Approach Delay, s/veh / LOS				17.2	B		18.4	B	110.7	F	0.0								
Intersection Delay, s/veh / LOS						51.5				D									
Multimodal Results				EB		WB		NB		SB									
Pedestrian LOS Score / LOS				2.40	B	1.91	B	2.78	C	2.49	B								
Bicycle LOS Score / LOS				1.34	A	1.00	A	1.25	A										

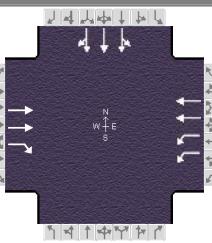
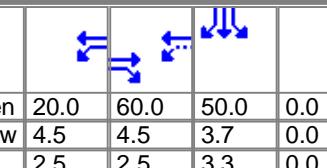
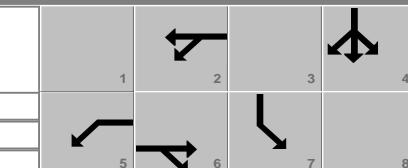
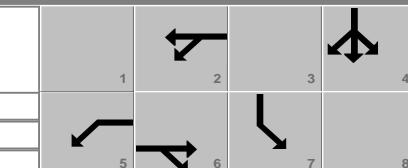
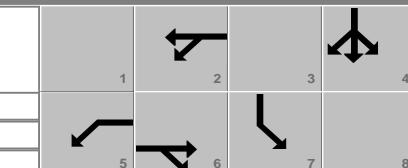
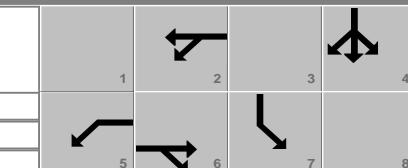
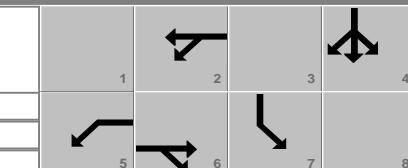
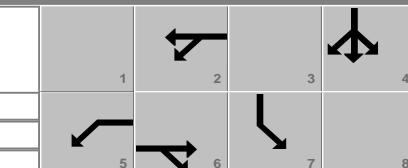
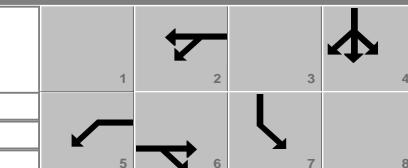
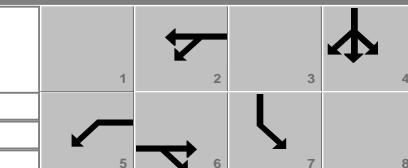
# HCS7 Signalized Intersection Results Summary

General Information						Intersection Information					
Agency		GFY				Duration, h		1.00			
Analyst		MDS		Analysis Date		3/29/2018		Area Type	Other		
Jurisdiction		City of Tampa		Time Period		AM		PHF	1.00		
Urban Street		State Road 60		Analysis Year		2018		Analysis Period	1> 7:00		
Intersection		21st Street and State R...		File Name		22nd-21st-SR60-EX-AM.xus					
Project Description		Clark Street Distribution Center									
Demand Information				EB		WB		NB		SB	
Approach Movement				L	T	R	L	T	R	L	
Demand ( v ), veh/h				376	188	144	1560			156	
										672	
										108	
Signal Information											
Cycle, s	151.0	Reference Phase	2								
Offset, s	15	Reference Point	Begin								
Uncoordinated	No	Simult. Gap E/W	Off		Green	20.0	60.0	50.0	0.0	0.0	
Force Mode	Fixed	Simult. Gap N/S	On		Yellow	4.5	4.5	3.7	0.0	0.0	
					Red	2.5	2.5	3.3	0.0	0.0	
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase					6	5	2				4
Case Number					7.3	1.0	4.0				12.0
Phase Duration, s					67.0	27.0	94.0				57.0
Change Period, ( Y+R <sub>c</sub> ), s					7.0	7.0	7.0				7.0
Max Allow Headway ( MAH ), s					0.0	3.1	0.0				4.1
Queue Clearance Time ( g <sub>s</sub> ), s						6.3					24.6
Green Extension Time ( g <sub>e</sub> ), s					0.0	0.2	0.0				3.5
Phase Call Probability						1.00					1.00
Max Out Probability						0.00					0.01
Movement Group Results				EB		WB		NB		SB	
Approach Movement				L	T	R	L	T	R	L	
Assigned Movement				6	16	5	2			7	
Adjusted Flow Rate ( v ), veh/h				376	100	142	1537			329	
Adjusted Saturation Flow Rate ( s ), veh/h/ln				1766	1585	1264	1752			1798	
Queue Service Time ( g <sub>s</sub> ), s				10.8	6.1	4.3	42.7			20.1	
Cycle Queue Clearance Time ( g <sub>c</sub> ), s				10.8	6.1	4.3	42.7			20.3	
Green Ratio ( g/C )				0.40	0.40	0.54	0.58			0.33	
Capacity ( c ), veh/h				1404	630	909	2019			610	
Volume-to-Capacity Ratio ( X )				0.268	0.159	0.156	0.761			575	
Back of Queue ( Q ), ft/ln ( 50 th percentile)				121.7	62.4	43.4	392.8			248.2	
Back of Queue ( Q ), veh/ln ( 50 th percentile)				4.8	2.5	1.3	15.2			9.2	
Queue Storage Ratio ( RQ ) ( 50 th percentile)				0.00	0.00	0.00	0.00			0.00	
Uniform Delay ( d <sub>1</sub> ), s/veh				30.7	29.3	18.8	16.1			40.5	
Incremental Delay ( d <sub>2</sub> ), s/veh				0.5	0.5	0.3	2.2			3.2	
Initial Queue Delay ( d <sub>3</sub> ), s/veh				0.0	0.0	0.0	0.0			0.0	
Control Delay ( d ), s/veh				31.2	29.8	19.1	18.3			43.7	
Level of Service (LOS)				C	C	B	B			D	
Approach Delay, s/veh / LOS				30.9	C	18.4	B	0.0		44.1	
Intersection Delay, s/veh / LOS						28.0			C		
Multimodal Results				EB		WB		NB		SB	
Pedestrian LOS Score / LOS				1.93	B	1.92	B	2.49	B	2.46	
Bicycle LOS Score / LOS				0.88	A	1.89	B			A	

# HCS7 Signalized Intersection Results Summary

General Information							Intersection Information														
Agency	GFY			Duration, h	1.00																
Analyst	MDS		Analysis Date	Apr 19, 2018		Area Type	Other														
Jurisdiction	City of Tampa		Time Period	PM		PHF	1.00														
Urban Street	State Road 60		Analysis Year	2018		Analysis Period	1> 7:00														
Intersection	21st Street and State R...			File Name	22nd-21st-SR60-EX-PM.xus																
Project Description	Clark Street Distribution Center																				
Demand Information				EB		WB		NB		SB											
Approach Movement				L	T	R	L	T	R	L	T	R									
Demand ( v ), veh/h				928	760	224	1228			208	1012	28									
Signal Information																					
Cycle, s	151.0	Reference Phase	2																		
Offset, s	90	Reference Point	Begin	Green	20.0	60.0	50.0	0.0	0.0												
Uncoordinated	No	Simult. Gap E/W	Off	Yellow	4.5	4.5	3.7	0.0	0.0												
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.5	2.5	3.3	0.0	0.0												
Timer Results				EBL		EBT		WBL		WBT		NBL		NBT		SBL		SBT			
Assigned Phase						6		5		2						4					
Case Number						7.3		1.0		4.0						12.0					
Phase Duration, s						67.0		27.0		94.0						57.0					
Change Period, ( Y+R_c ), s						7.0		7.0		7.0						7.0					
Max Allow Headway ( MAH ), s						0.0		3.1		0.0						4.1					
Queue Clearance Time ( g_s ), s								8.2								33.1					
Green Extension Time ( g_e ), s						0.0		0.5		0.0						4.7					
Phase Call Probability								1.00								1.00					
Max Out Probability						0.00										0.15					
Movement Group Results				EB			WB			NB			SB								
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R						
Assigned Movement				6	16	5	2						7	4	14						
Adjusted Flow Rate ( v ), veh/h				928	484	262	1439						431	404	401						
Adjusted Saturation Flow Rate ( s ), veh/h/ln				1781	1598	1606	1781						1831	1885	1872						
Queue Service Time ( g_s ), s				32.1	39.6	6.2	38.5						31.1	27.5	27.5						
Cycle Queue Clearance Time ( g_c ), s				32.1	39.6	6.2	38.5						31.1	27.5	27.5						
Green Ratio ( g/C )				0.40	0.40	0.54	0.58						0.33	0.33	0.33						
Capacity ( c ), veh/h				1415	635	728	2052						606	624	620						
Volume-to-Capacity Ratio ( X )				0.656	0.762	0.361	0.701						0.711	0.647	0.647						
Back of Queue ( Q ), ft/ln ( 50 th percentile)				361.4	423.1	64.7	365.1						385.2	348.5	343.8						
Back of Queue ( Q ), veh/ln ( 50 th percentile)				14.2	16.8	2.4	14.4						15.4	13.8	13.8						
Queue Storage Ratio ( RQ ) ( 50 th percentile)				0.00	0.00	0.00	0.00						0.00	0.00	0.00						
Uniform Delay ( d_1 ), s/veh				37.1	39.3	22.3	17.4						44.2	43.0	43.0						
Incremental Delay ( d_2 ), s/veh				2.4	8.9	1.1	1.7						7.2	5.2	5.3						
Initial Queue Delay ( d_3 ), s/veh				0.0	0.0	0.0	0.0						0.0	0.0	0.0						
Control Delay ( d ), s/veh				39.5	48.3	23.4	19.0						51.4	48.2	48.3						
Level of Service (LOS)				D	D	C	B						D	D	D						
Approach Delay, s/veh / LOS				42.5	D	19.7	B			0.0			49.3	D							
Intersection Delay, s/veh / LOS				35.5				D													
Multimodal Results				EB			WB			NB			SB								
Pedestrian LOS Score / LOS				1.93	B	1.99	B	2.49	B	2.72	C										
Bicycle LOS Score / LOS				1.65	B	1.69	B			1.17	A										

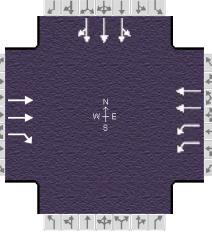
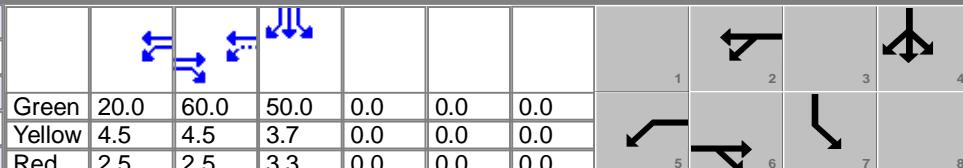
# HCS7 Signalized Intersection Results Summary

General Information						Intersection Information												
Agency	GFY					Duration, h	1.00											
Analyst	MDS		Analysis Date	3/29/2018		Area Type	Other											
Jurisdiction	City of Tampa			Time Period	AM		PHF	1.00										
Urban Street	State Road 60			Analysis Year	2018		Analysis Period	1 > 7:00										
Intersection	21st Street and State R...			File Name	22nd-21st-SR60-FB-AM.xus													
Project Description	Clark Street Distribution Center																	
Demand Information				EB		WB		NB		SB								
Approach Movement				L	T	R	L	T	R	L	T	R						
Demand ( v ), veh/h				392	196	150	1624			163	700	113						
Signal Information																		
Cycle, s	151.0	Reference Phase	2															
Offset, s	15	Reference Point	Begin															
Uncoordinated	No	Simult. Gap E/W	Off															
Force Mode	Fixed	Simult. Gap N/S	On															
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT							
Assigned Phase					6	5	2				4							
Case Number					7.3	1.0	4.0				12.0							
Phase Duration, s					67.0	27.0	94.0				57.0							
Change Period, ( Y+R <sub>c</sub> ), s					7.0	7.0	7.0				7.0							
Max Allow Headway ( MAH ), s					0.0	3.1	0.0				4.1							
Queue Clearance Time ( g <sub>s</sub> ), s						6.4					25.8							
Green Extension Time ( g <sub>e</sub> ), s					0.0	0.2	0.0				3.7							
Phase Call Probability						1.00					1.00							
Max Out Probability						0.00					0.01							
Movement Group Results				EB		WB		NB		SB								
Approach Movement				L	T	R	L	T	R	L	T	R						
Assigned Movement				6	16	5	2			7	4	14						
Adjusted Flow Rate ( v ), veh/h				392	104	143	1552			342	318	302						
Adjusted Saturation Flow Rate ( s ), veh/h/ln				1766	1585	1264	1752			1798	1841	1737						
Queue Service Time ( g <sub>s</sub> ), s				11.4	6.4	4.4	44.6			23.8	21.1	21.3						
Cycle Queue Clearance Time ( g <sub>c</sub> ), s				11.4	6.4	4.4	44.6			23.8	21.1	21.3						
Green Ratio ( g/C )				0.40	0.40	0.54	0.58			0.33	0.33	0.33						
Capacity ( c ), veh/h				1404	630	897	2019			595	610	575						
Volume-to-Capacity Ratio ( X )				0.279	0.165	0.160	0.769			0.575	0.522	0.525						
Back of Queue ( Q ), ft/ln ( 50 th percentile)				127.5	65.2	43.9	418.8			280.6	261.5	242						
Back of Queue ( Q ), veh/ln ( 50 th percentile)				5.0	2.6	1.4	16.2			11.2	10.1	9.7						
Queue Storage Ratio ( RQ ) ( 50 th percentile)				0.00	0.00	0.00	0.00			0.00	0.00	0.00						
Uniform Delay ( d <sub>1</sub> ), s/veh				30.8	29.3	19.0	17.2			41.7	40.8	40.9						
Incremental Delay ( d <sub>2</sub> ), s/veh				0.5	0.6	0.3	2.1			4.1	3.2	3.5						
Initial Queue Delay ( d <sub>3</sub> ), s/veh				0.0	0.0	0.0	0.0			0.0	0.0	0.0						
Control Delay ( d ), s/veh				31.3	29.9	19.2	19.3			45.8	44.1	44.3						
Level of Service (LOS)				C	C	B	B			D	D	D						
Approach Delay, s/veh / LOS				31.0	C	19.3	B	0.0		44.8		D						
Intersection Delay, s/veh / LOS						28.9			C									
Multimodal Results				EB		WB		NB		SB								
Pedestrian LOS Score / LOS				1.93	B	1.92	B	2.49	B	2.47	B							
Bicycle LOS Score / LOS				0.90	A	1.95	B			1.02	A							

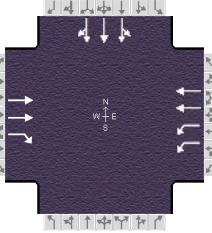
# HCS7 Signalized Intersection Results Summary

General Information						Intersection Information								
Agency		GFY						Duration, h	1.00					
Analyst		MDS		Analysis Date		Apr 19, 2018		Area Type	Other					
Jurisdiction		City of Tampa		Time Period		PM		PHF	1.00					
Urban Street		State Road 60		Analysis Year		2018		Analysis Period	1> 7:00					
Intersection		21st Street and State R...			File Name		22nd-21st-SR60-FB-PM.xus							
Project Description		Clark Street Distribution Center												
Demand Information				EB		WB		NB		SB				
Approach Movement				L	T	R	L	T	R	L	T	R		
Demand ( v ), veh/h				966	791	234	1278			217	1053	30		
Signal Information														
Cycle, s	151.0	Reference Phase	2											
Offset, s	90	Reference Point	Begin	Green	20.0	60.0	50.0	0.0	0.0	0.0				
Uncoordinated	No	Simult. Gap E/W	Off	Yellow	4.5	4.5	3.7	0.0	0.0	0.0				
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.5	2.5	3.3	0.0	0.0	0.0				
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT			
Assigned Phase						6	5	2			4			
Case Number						7.3	1.0	4.0			12.0			
Phase Duration, s						67.0	27.0	94.0			57.0			
Change Period, ( Y+R <sub>c</sub> ), s						7.0	7.0	7.0			7.0			
Max Allow Headway ( MAH ), s						0.0	3.1	0.0			4.1			
Queue Clearance Time ( g <sub>s</sub> ), s							8.3				34.8			
Green Extension Time ( g <sub>e</sub> ), s						0.0	0.5	0.0			4.7			
Phase Call Probability							1.00				1.00			
Max Out Probability							0.00				0.22			
Movement Group Results				EB		WB		NB		SB				
Approach Movement				L	T	R	L	T	R	L	T	R		
Assigned Movement				6	16	5	2			7	4	14		
Adjusted Flow Rate ( v ), veh/h				966	503	270	1475			449	421	418		
Adjusted Saturation Flow Rate ( s ), veh/h/ln				1781	1598	1606	1781			1831	1885	1871		
Queue Service Time ( g <sub>s</sub> ), s				33.9	41.8	6.3	40.9			32.8	29.0	29.0		
Cycle Queue Clearance Time ( g <sub>c</sub> ), s				33.9	41.8	6.3	40.9			32.8	29.0	29.0		
Green Ratio ( g/C )				0.40	0.40	0.54	0.58			0.33	0.33	0.33		
Capacity ( c ), veh/h				1415	635	708	2052			606	624	620		
Volume-to-Capacity Ratio ( X )				0.683	0.792	0.382	0.719			0.741	0.674	0.674		
Back of Queue ( Q ), ft/ln ( 50 th percentile)				383	452.4	66.8	392.6			409.2	369	363.6		
Back of Queue ( Q ), veh/ln ( 50 th percentile)				15.1	18.0	2.5	15.5			16.4	14.6	14.5		
Queue Storage Ratio ( RQ ) ( 50 th percentile)				0.00	0.00	0.00	0.00			0.00	0.00	0.00		
Uniform Delay ( d <sub>1</sub> ), s/veh				37.6	40.0	23.0	18.2			44.8	43.5	43.5		
Incremental Delay ( d <sub>2</sub> ), s/veh				2.7	10.5	1.2	1.8			8.3	5.9	5.9		
Initial Queue Delay ( d <sub>3</sub> ), s/veh				0.0	0.0	0.0	0.0			0.0	0.0	0.0		
Control Delay ( d ), s/veh				40.4	50.5	24.2	19.9			53.1	49.4	49.4		
Level of Service (LOS)				D	D	C	B			D	D	D		
Approach Delay, s/veh / LOS				43.8	D	20.6	C	0.0		50.7	D			
Intersection Delay, s/veh / LOS						36.8			D					
Multimodal Results				EB		WB		NB		SB				
Pedestrian LOS Score / LOS				1.93	B	1.99	B	2.49	B	2.74	C			
Bicycle LOS Score / LOS				1.70	B	1.74	B			1.20	A			

# HCS7 Signalized Intersection Results Summary

General Information						Intersection Information													
Agency	GFY			Duration, h															
Analyst	MDS		Analysis Date	3/29/2018		Area Type													
Jurisdiction	City of Tampa		Time Period	AM		PHF													
Urban Street	State Road 60		Analysis Year	2018		Analysis Period													
Intersection	21st Street and State R...			File Name		22nd-21st-SR60-FBP-AM.xus													
Project Description	Clark Street Distribution Center																		
Demand Information				EB		WB		NB		SB									
Approach Movement				L	T	R	L	T	R	L	T								
Demand ( v ), veh/h				392	196	151	1625			163	700								
Signal Information																			
Cycle, s	151.0	Reference Phase	2																
Offset, s	15	Reference Point	Begin	Green	20.0	60.0	50.0	0.0	0.0	0.0	0.0								
Uncoordinated	No	Simult. Gap E/W	Off	Yellow	4.5	4.5	3.7	0.0	0.0	0.0	0.0								
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.5	2.5	3.3	0.0	0.0	0.0	0.0								
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT								
Assigned Phase					6	5	2				4								
Case Number					7.3	1.0	4.0				12.0								
Phase Duration, s					67.0	27.0	94.0				57.0								
Change Period, ( Y+R <sub>c</sub> ), s					7.0	7.0	7.0				7.0								
Max Allow Headway ( MAH ), s					0.0	3.1	0.0				4.1								
Queue Clearance Time ( g <sub>s</sub> ), s						6.4					25.8								
Green Extension Time ( g <sub>e</sub> ), s					0.0	0.2	0.0				3.7								
Phase Call Probability						1.00					1.00								
Max Out Probability						0.00					0.01								
Movement Group Results				EB		WB		NB		SB									
Approach Movement				L	T	R	L	T	R	L	T								
Assigned Movement				6	16	5	2			7	4								
Adjusted Flow Rate ( v ), veh/h				392	104	144	1548			342	318								
Adjusted Saturation Flow Rate ( s ), veh/h/ln				1766	1585	1250	1752			1798	1841								
Queue Service Time ( g <sub>s</sub> ), s				11.4	6.4	4.4	44.5			23.8	21.1								
Cycle Queue Clearance Time ( g <sub>c</sub> ), s				11.4	6.4	4.4	44.5			23.8	21.1								
Green Ratio ( g/C )				0.40	0.40	0.54	0.58			0.33	0.33								
Capacity ( c ), veh/h				1404	630	888	2019			595	610								
Volume-to-Capacity Ratio ( X )				0.279	0.165	0.162	0.767			0.575	0.522								
Back of Queue ( Q ), ft/ln ( 50 th percentile)				127.5	65.2	44.4	418.7			280.6	261.5								
Back of Queue ( Q ), veh/ln ( 50 th percentile)				5.0	2.6	1.4	16.2			11.2	10.1								
Queue Storage Ratio ( RQ ) ( 50 th percentile)				0.00	0.00	0.00	0.00			0.00	0.00								
Uniform Delay ( d <sub>1</sub> ), s/veh				30.8	29.3	19.0	17.3			41.7	40.8								
Incremental Delay ( d <sub>2</sub> ), s/veh				0.5	0.6	0.3	2.1			4.1	3.2								
Initial Queue Delay ( d <sub>3</sub> ), s/veh				0.0	0.0	0.0	0.0			0.0	0.0								
Control Delay ( d ), s/veh				31.3	29.9	19.3	19.3			45.8	44.1								
Level of Service (LOS)				C	C	B	B			D	D								
Approach Delay, s/veh / LOS				31.0	C	19.3	B	0.0		44.8	D								
Intersection Delay, s/veh / LOS						28.9			C										
Multimodal Results				EB		WB		NB		SB									
Pedestrian LOS Score / LOS				1.93	B	1.92	B	2.49	B	2.47	B								
Bicycle LOS Score / LOS				0.90	A	1.95	B			1.02	A								

# HCS7 Signalized Intersection Results Summary

General Information							Intersection Information														
Agency	GFY			Duration, h	1.00																
Analyst	MDS		Analysis Date	Apr 19, 2018		Area Type	Other														
Jurisdiction	City of Tampa		Time Period	PM		PHF	1.00														
Urban Street	State Road 60		Analysis Year	2018		Analysis Period	1> 7:00														
Intersection	21st Street and State R...			File Name	22nd-21st-SR60-FBP-PM.xus																
Project Description	Clark Street Distribution Center																				
Demand Information				EB		WB		NB		SB											
Approach Movement				L	T	R	L	T	R	L	T	R									
Demand ( v ), veh/h				966	791	234	1278			217	1053	30									
Signal Information																					
Cycle, s	151.0	Reference Phase	2																		
Offset, s	90	Reference Point	Begin																		
Uncoordinated	No	Simult. Gap E/W	Off	Green	20.0	60.0	50.0	0.0	0.0	1	2	3									
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	4.5	4.5	3.7	0.0	0.0	5	6	7									
				Red	2.5	2.5	3.3	0.0	0.0	8											
Timer Results				EBL		EBT		WBL		WBT		NBL		NBT		SBL		SBT			
Assigned Phase						6		5		2						4					
Case Number						7.3		1.0		4.0						12.0					
Phase Duration, s						67.0		27.0		94.0						57.0					
Change Period, ( Y+R <sub>c</sub> ), s						7.0		7.0		7.0						7.0					
Max Allow Headway ( MAH ), s						0.0		3.1		0.0						4.1					
Queue Clearance Time ( g <sub>s</sub> ), s								8.3								34.8					
Green Extension Time ( g <sub>e</sub> ), s						0.0		0.5		0.0						4.7					
Phase Call Probability								1.00								1.00					
Max Out Probability						0.00										0.22					
Movement Group Results				EB			WB			NB			SB								
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R						
Assigned Movement				6	16	5	2						7	4	14						
Adjusted Flow Rate ( v ), veh/h				966	503	270	1475						449	421	418						
Adjusted Saturation Flow Rate ( s ), veh/h/ln				1781	1598	1606	1781						1831	1885	1871						
Queue Service Time ( g <sub>s</sub> ), s				33.9	41.8	6.3	40.9						32.8	29.0	29.0						
Cycle Queue Clearance Time ( g <sub>c</sub> ), s				33.9	41.8	6.3	40.9						32.8	29.0	29.0						
Green Ratio ( g/C )				0.40	0.40	0.54	0.58						0.33	0.33	0.33						
Capacity ( c ), veh/h				1415	635	708	2052						606	624	620						
Volume-to-Capacity Ratio ( X )				0.683	0.792	0.382	0.719						0.741	0.674	0.674						
Back of Queue ( Q ), ft/ln ( 50 th percentile)				383	452.4	66.8	392.6						409.2	369	363.6						
Back of Queue ( Q ), veh/ln ( 50 th percentile)				15.1	18.0	2.5	15.5						16.4	14.6	14.5						
Queue Storage Ratio ( RQ ) ( 50 th percentile)				0.00	0.00	0.00	0.00						0.00	0.00	0.00						
Uniform Delay ( d <sub>1</sub> ), s/veh				37.6	40.0	23.0	18.2						44.8	43.5	43.5						
Incremental Delay ( d <sub>2</sub> ), s/veh				2.7	10.5	1.2	1.8						8.3	5.9	5.9						
Initial Queue Delay ( d <sub>3</sub> ), s/veh				0.0	0.0	0.0	0.0						0.0	0.0	0.0						
Control Delay ( d ), s/veh				40.4	50.5	24.2	19.9						53.1	49.4	49.4						
Level of Service (LOS)				D	D	C	B						D	D	D						
Approach Delay, s/veh / LOS				43.8	D	20.6	C			0.0			50.7	D							
Intersection Delay, s/veh / LOS				36.8						D											
Multimodal Results				EB			WB			NB			SB								
Pedestrian LOS Score / LOS				1.93	B	1.99	B			2.49	B		2.74	C							
Bicycle LOS Score / LOS				1.70	B	1.74	B						1.20	A							